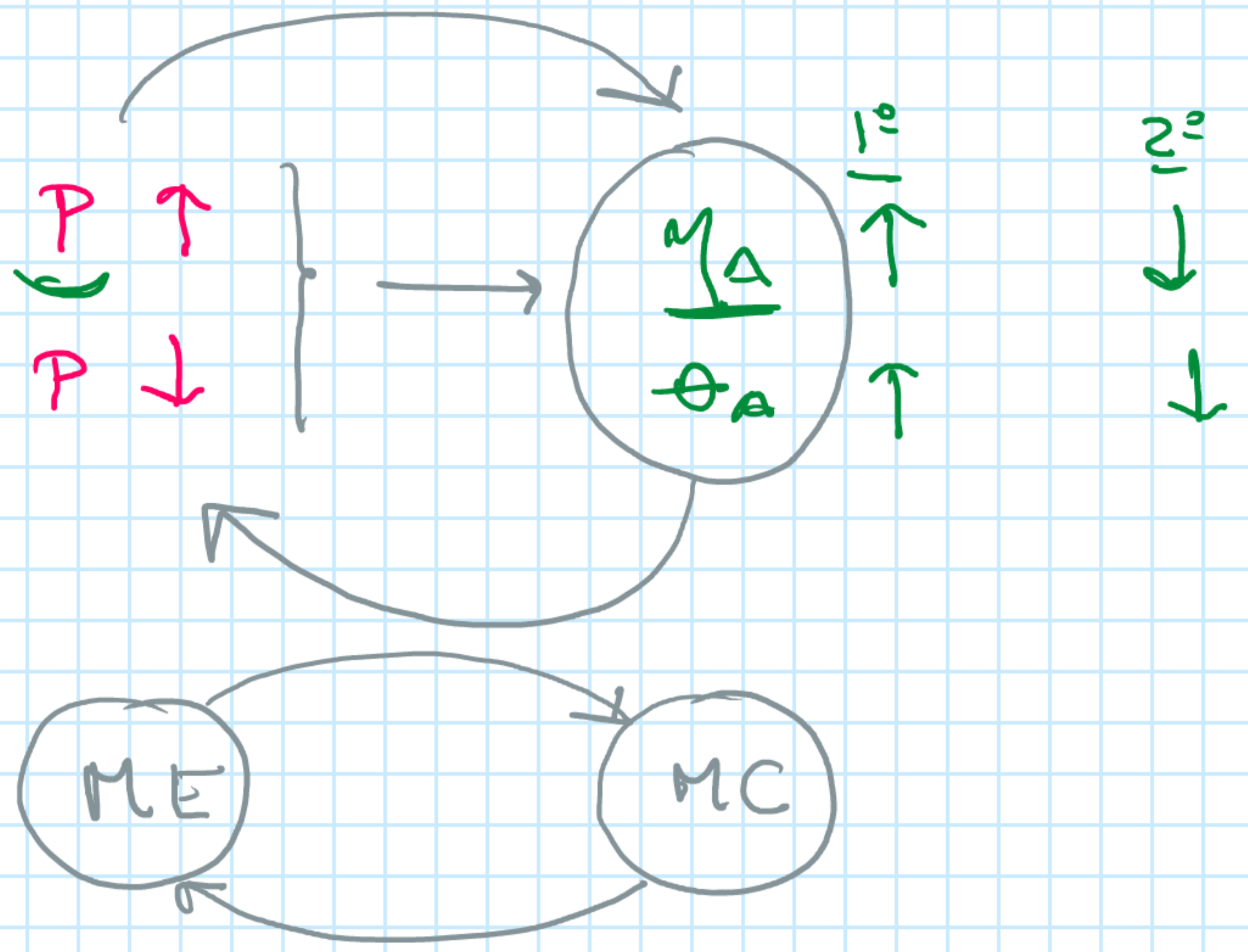


$\frac{1}{EI}$

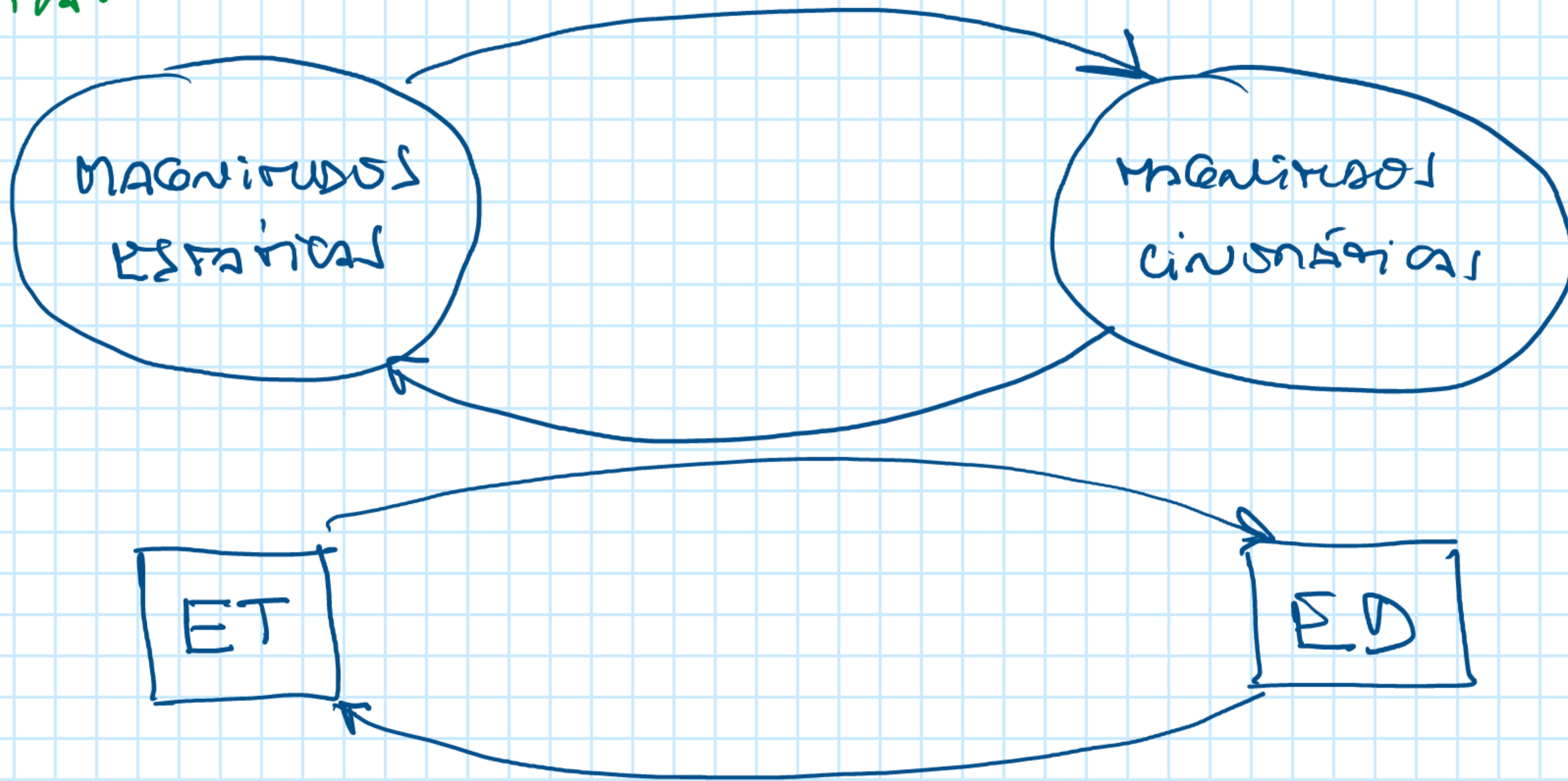


$$\begin{Bmatrix} \delta_A \\ \theta_A \end{Bmatrix}_{AC}$$

$$\begin{Bmatrix} \delta_A \\ \theta_A \end{Bmatrix}_M$$

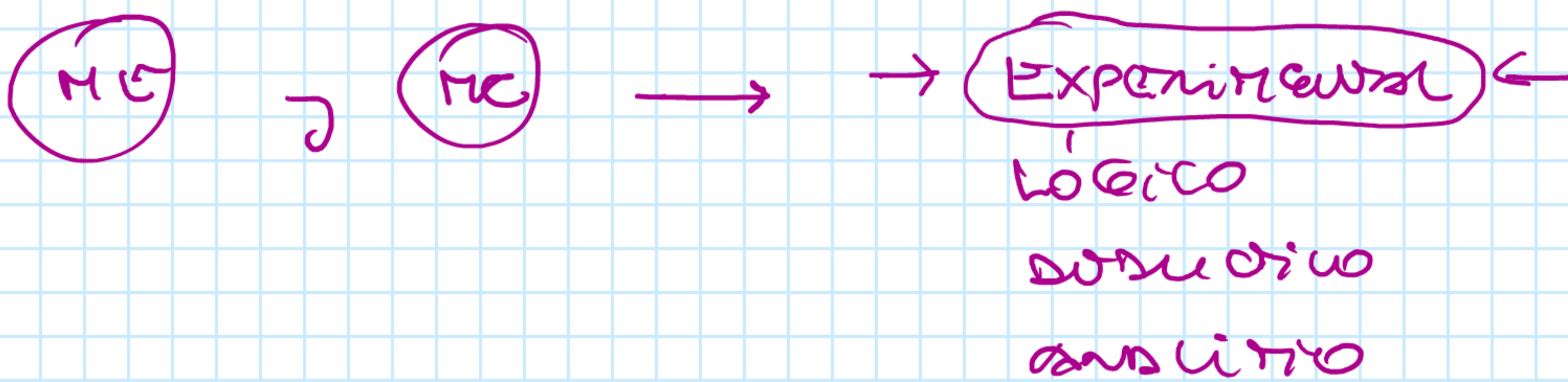
$$\begin{Bmatrix} \delta_A \\ \theta_A \end{Bmatrix}_{H^0}$$

$$\begin{Bmatrix} \delta_A \\ \theta_A \end{Bmatrix}_{\text{Corta}}$$



$$\left. \begin{array}{l} ET \rightarrow [TT] \\ ED \rightarrow [TD] \end{array} \right\} \rightarrow \text{lógico - analítico - deductivo.}$$

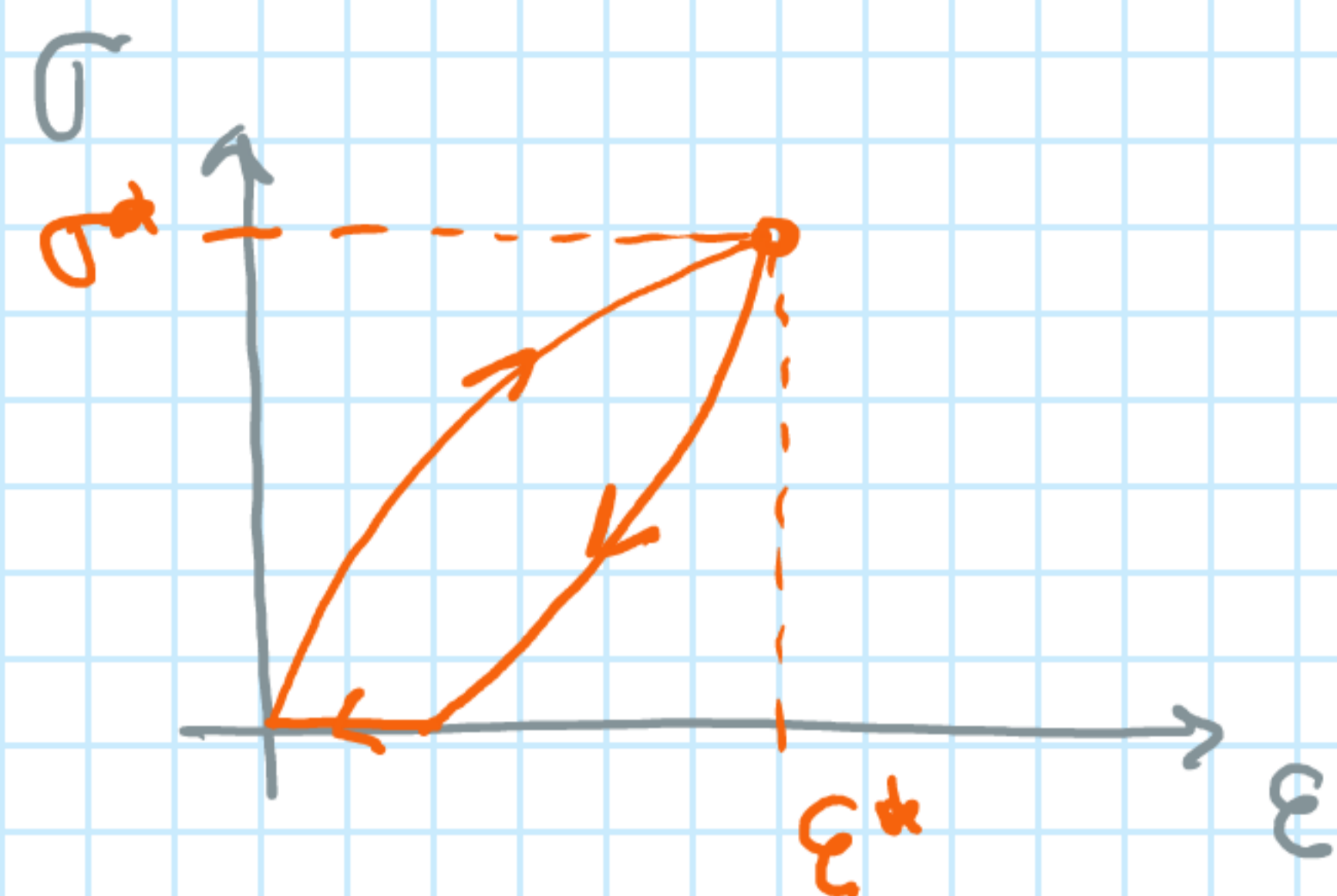
EL CAMINO PUEDE USAR O SOBREPONER P/ ENCONTRAR ESTE VÍNCULO COMO



"RELACIONES ENTRE TENSIONES Y DEFORMACIONES"

- MATERIALES  $\rightarrow$  HOMOGENEOS / ISOTROPAS.

- PERIODO DE COMPORTEMIENTO DEL MATERIAL:  $\rightarrow$  ELÁSTICO LINEAL.



- ME  $\rightarrow$  ET  $\rightarrow$  [TE]  $\rightarrow$  simétrica 3x3  $\rightarrow$  6 valores distintos.  $\rightarrow$   $\left\{ \begin{matrix} \bar{\sigma} \\ 6 \times 1 \end{matrix} \right\}$
- MC  $\rightarrow$  ED  $\rightarrow$  [TD]  $\rightarrow$  " "  $\rightarrow$  6 " "  $\rightarrow$   $\left\{ \begin{matrix} \bar{\epsilon}_d \\ 6 \times 1 \end{matrix} \right\}$

$$\left\{ \bar{\epsilon}_d \right\} = [B] \left\{ \bar{\sigma} \right\}$$

$\uparrow$   $\uparrow$   $\uparrow$   
 [TD]  $\uparrow$  [TE]  
 6x1  $\uparrow$  6x6  $\uparrow$  6x1

$$\begin{pmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{pmatrix} = \begin{bmatrix} a_{11} & \dots & \dots & \dots & \dots & a_{16} \\ \vdots & & & & & \vdots \\ a_{61} & \dots & \dots & \dots & \dots & a_{66} \end{bmatrix} \begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{pmatrix}$$

6x1  $\quad$  6x6  $\quad$  6x1.

36 ctes.  
 $\downarrow$   
 21 ctes.  
 $\downarrow$   
 18 ctes.  
 $\downarrow$   
 3 ctes.  
 $\downarrow$   
 2 ctes.

DE LA REALIZACIÓN DE ENSAYOS Y EXPERIMENTOS DE LABORATORIO:

(I) Por esta forma de periodo elástico  $\rightarrow$  se cumple la "LEY DE BETTI"  $\rightarrow$  LOS DE RECIPROCIDAD:

$$a_{ij} = a_{ji} \rightarrow [B] \rightarrow \text{simétrica.}$$

(II) En caso isotropo y homogéneo:

$\rightarrow$  II.a  $\rightarrow$  Las deformaciones elásticas longitudinales son independientes de las tensiones tangenciales  $\tau$ .  
 $\epsilon_{ii}$  y  $\tau_{ij}$  están desacopladas

$\rightarrow$  II.b  $\rightarrow$  Las deform. espaciales transversales y las tensiones normales son independientes.  
 $\tau_{ij}$  y  $\tau_{ii}$   $\rightarrow$  están desacopladas.

LA EXPRESIÓN MATEMÁTICA ASES PUEDE

$$\begin{pmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{pmatrix} = \begin{bmatrix} a_{11} & a_{12} & a_{13} & & & \\ \vdots & \vdots & \vdots & & & 0 \\ a_{31} & \dots & a_{33} & & & \\ \vdots & & & a_{44} & \dots & a_{46} \\ \vdots & & & \vdots & & \vdots \\ a_{64} & \dots & a_{66} & & & \end{bmatrix} \begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{pmatrix}$$

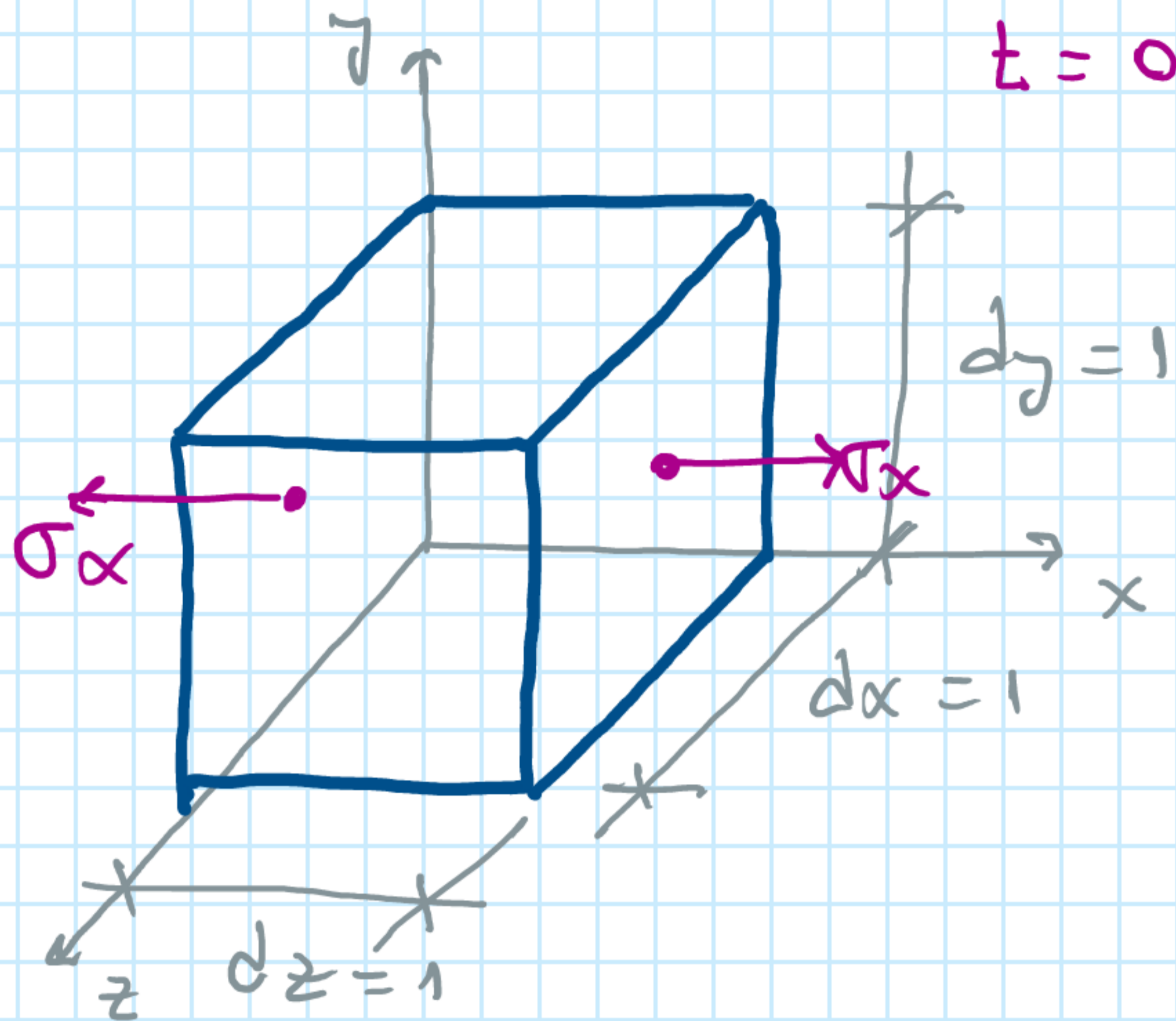
6x1  $\quad$  6x6  $\quad$  6x1.

$$\left\{ \bar{\epsilon}_d \right\} = [B] \left\{ \bar{\sigma} \right\} \Leftrightarrow \left\{ \bar{\sigma} \right\} = [E] \left\{ \bar{\epsilon}_d \right\}$$

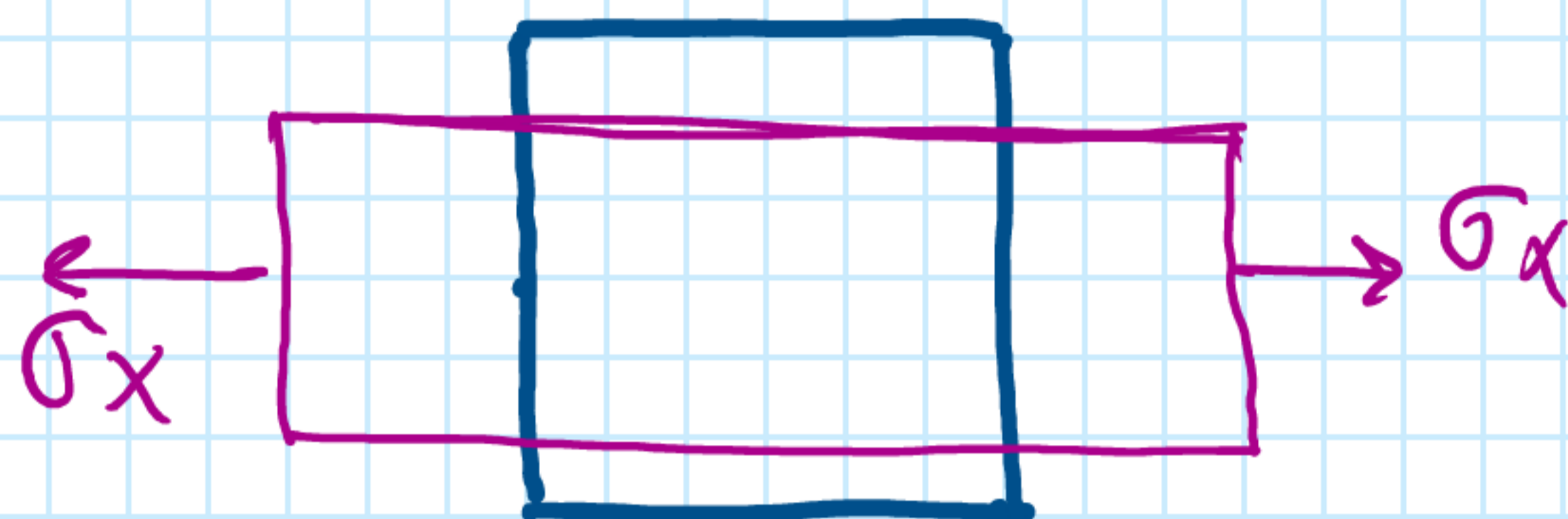
**1º ENSAYO**

$t=0 \rightarrow$  ~~condiciones anteriores~~

$t=0^+ \rightarrow$  nuevas condiciones



$$F_x = \sigma_x dA = \sigma_x \cdot \underbrace{dx}_{=1} \cdot \underbrace{dy}_{=1} = \sigma_x$$



$$\epsilon_{ii} = \frac{\Delta L_{ii}}{L_{0,ii}} \rightarrow \epsilon_{ii} = \Delta L_{ii}$$

**1º caso**

$\sigma_x \neq 0$     $\sigma_y = \sigma_z = 0$     $\sigma_x > 0$

$$\epsilon_{xx} = \frac{\sigma_x}{E}$$

$\sigma_x > 0 \rightarrow \epsilon_{xx} > 0$

$\epsilon_{yy} < 0 \wedge \epsilon_{zz} < 0$

$$\epsilon_{yy} = -\underbrace{\mu_{yx}} \cdot \underbrace{\epsilon_{xx}} = -\mu_{yx} \cdot \frac{\sigma_x}{E}$$

$\mu_{yx} \wedge \mu_{zx}$  se denominan  $\nu$

$$\epsilon_{zz} = -\mu_{zx} \cdot \epsilon_{xx} = -\mu_{zx} \cdot \frac{\sigma_x}{E}$$

Nota:

$\mu_{ij}$   $\rightarrow$  coeficiente de Poisson

$\mu_{ij}$   $\rightarrow$  coeficiente de Poisson

**2º caso**

$\sigma_y \neq 0$     $\sigma_x = \sigma_z = 0$     $\sigma_y > 0$

$$\begin{cases} \epsilon_{yy} = \frac{\sigma_y}{E} \\ \epsilon_{xx} = -\mu_{xy} \cdot \epsilon_{yy} = -\mu_{xy} \cdot \frac{\sigma_y}{E} \\ \epsilon_{zz} = -\mu_{zy} \cdot \epsilon_{yy} = -\mu_{zy} \cdot \frac{\sigma_y}{E} \end{cases}$$

**3º caso**

$\sigma_z \neq 0$     $\sigma_x = \sigma_y = 0$     $\sigma_z > 0$

$$\begin{cases} \epsilon_{zz} = \frac{\sigma_z}{E} \\ \epsilon_{xx} = -\mu_{xz} \cdot \epsilon_{zz} = -\mu_{xz} \cdot \frac{\sigma_z}{E} \\ \epsilon_{yy} = -\mu_{yz} \cdot \epsilon_{zz} = -\mu_{yz} \cdot \frac{\sigma_z}{E} \end{cases}$$

MATERIALES ISÓTROPAS →

$$\mu_{yx} = \mu_{xy} = \mu_{xz} = \mu_{zx} = \mu_{yz} = \mu_{zy} = \mu \quad \text{COEFICIENTE DE POISSON}$$

$$\mu = - \frac{\epsilon_{\perp}}{\epsilon_{\parallel}}$$

← DIRECCIÓN ⊥ A LA DIRECCIÓN DE LA CARGA. (DEF. ESPECÍFICA LONGITUDINAL)  
 ← DIRECCIÓN DE APPLICACIÓN DE LA CARGA (DIRECCIÓN ESPECÍFICA LONGITUDINAL).

A  $\mu$  se lo define  $\oplus$  → se produce a la compresión del eje (-).

— 0 — 0 — 0 —

4º CASO  $\sigma_x \neq 0 \wedge \sigma_y \neq 0 \wedge \sigma_z \neq 0.$

Por la linealidad como los materiales → PSE.

$$\begin{cases} \epsilon_{xx} = \frac{\sigma_x}{E} - \mu \frac{\sigma_y}{E} - \mu \frac{\sigma_z}{E} = \frac{1}{E} [\sigma_x - \mu(\sigma_y + \sigma_z)] \\ \epsilon_{yy} = \frac{\sigma_y}{E} - \mu \frac{\sigma_x}{E} - \mu \frac{\sigma_z}{E} = \frac{1}{E} [\sigma_y - \mu(\sigma_x + \sigma_z)] \\ \epsilon_{zz} = \frac{\sigma_z}{E} - \mu \frac{\sigma_x}{E} - \mu \frac{\sigma_y}{E} = \frac{1}{E} [\sigma_z - \mu(\sigma_x + \sigma_y)] \end{cases}$$

"LEY GENERALIZADA DE HOOKE"

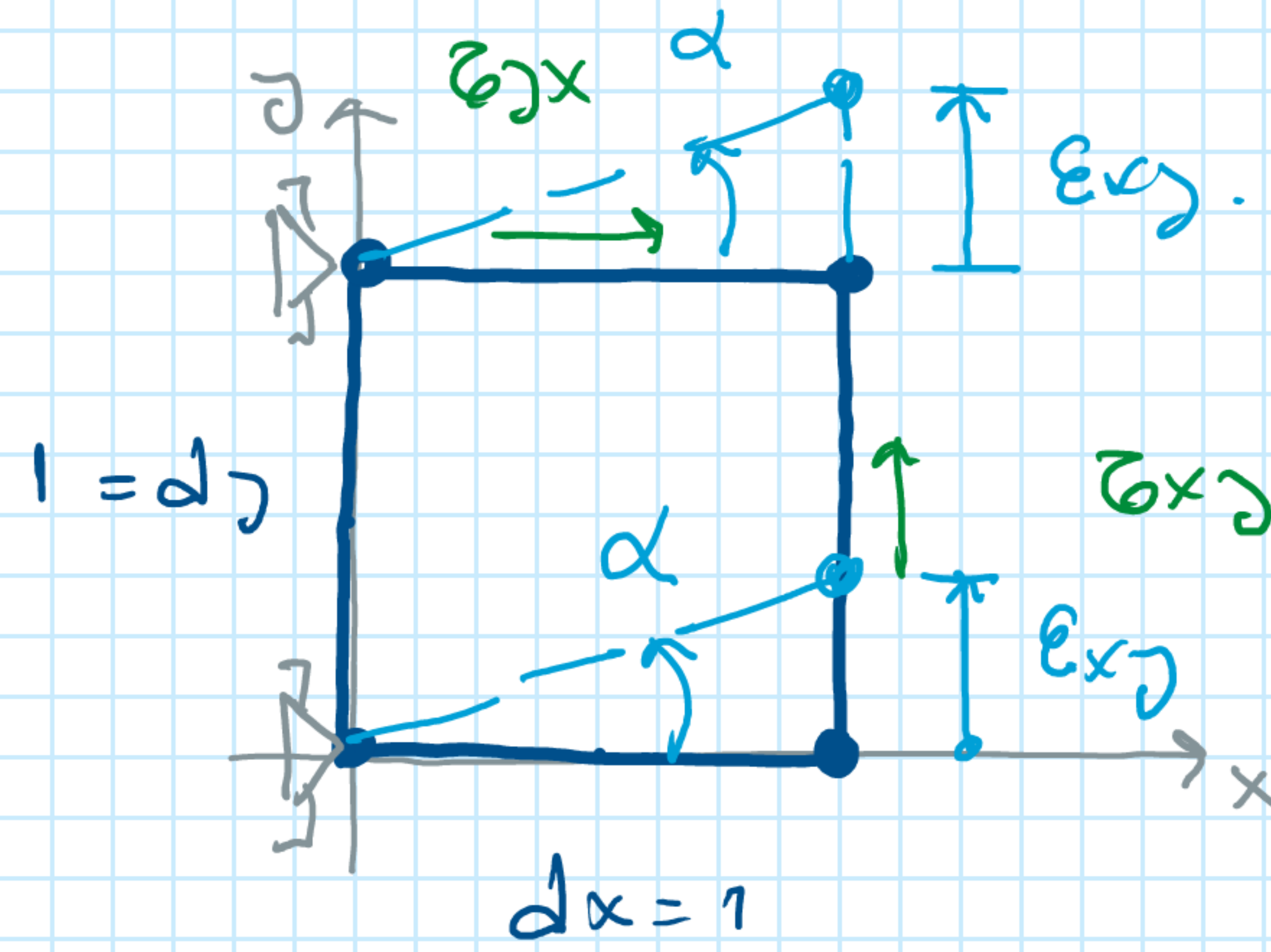
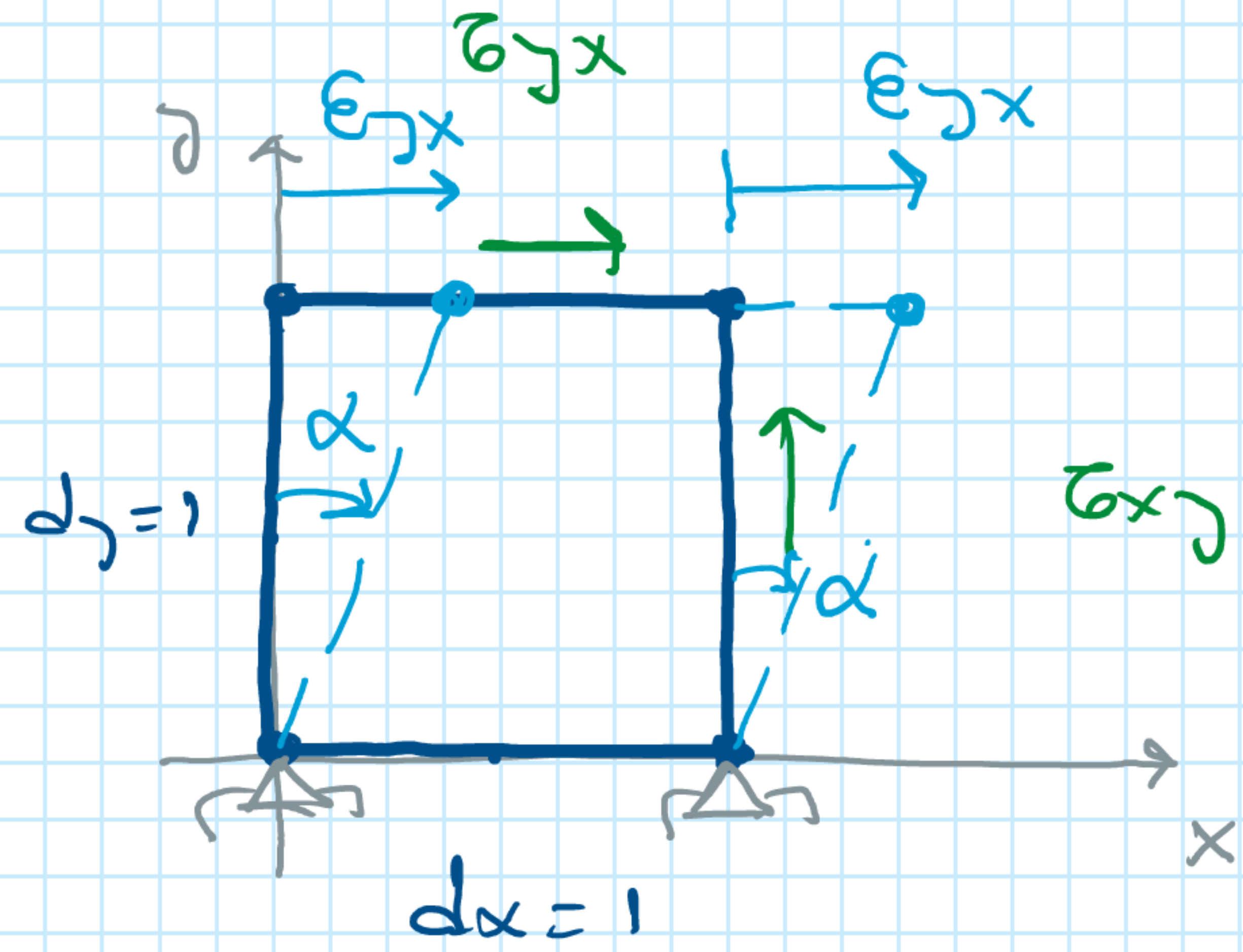
I)  $\mu$ : es igual que  $\nu$ ,  $\nu$  es una característica o propiedad del material.

II)  $\mu$  en materiales anisótropos u ortótropos son  $\neq$   
 $\mu_{ij} \neq \mu_{kl}$

$$\mu_{yx} = - \frac{\epsilon_{yy}}{\epsilon_{xx}} \quad \mu_{zy} = - \frac{\epsilon_{zz}}{\epsilon_{yy}}$$

$$\mu_{ij} = - \frac{\epsilon_{ii}}{\epsilon_{jj}} \quad \leftarrow \text{causa. ( ) inverso.}$$

**2º LEY DE HOOKE** → VINCULO A LAS LEYES TANGENCIALES con las deformaciones de POSICIONES TRANSVERSAS.



HIPOTESIS DE DEFORMACIONES DEFORMACIONES:

Arco  $\equiv$  Curva  $\equiv$  TANGENTE

$$\alpha \cdot \underbrace{dy}_{=1} = \epsilon_{yx} \rightarrow \boxed{\alpha = \epsilon_{yx}} \quad (1)$$

$$\alpha \cdot \underbrace{dx}_{=1} = \epsilon_{xy} \rightarrow \boxed{\alpha = \epsilon_{xy}} \quad (2)$$

$\alpha$  PARA AMBOS CASOS ES EL MISMO.

$$\left. \begin{array}{l} (1) \quad \alpha = \epsilon_{yx} \\ (2) \quad \alpha = \epsilon_{xy} \end{array} \right\} \rightarrow \boxed{\epsilon_{xy} = \epsilon_{yx}}$$

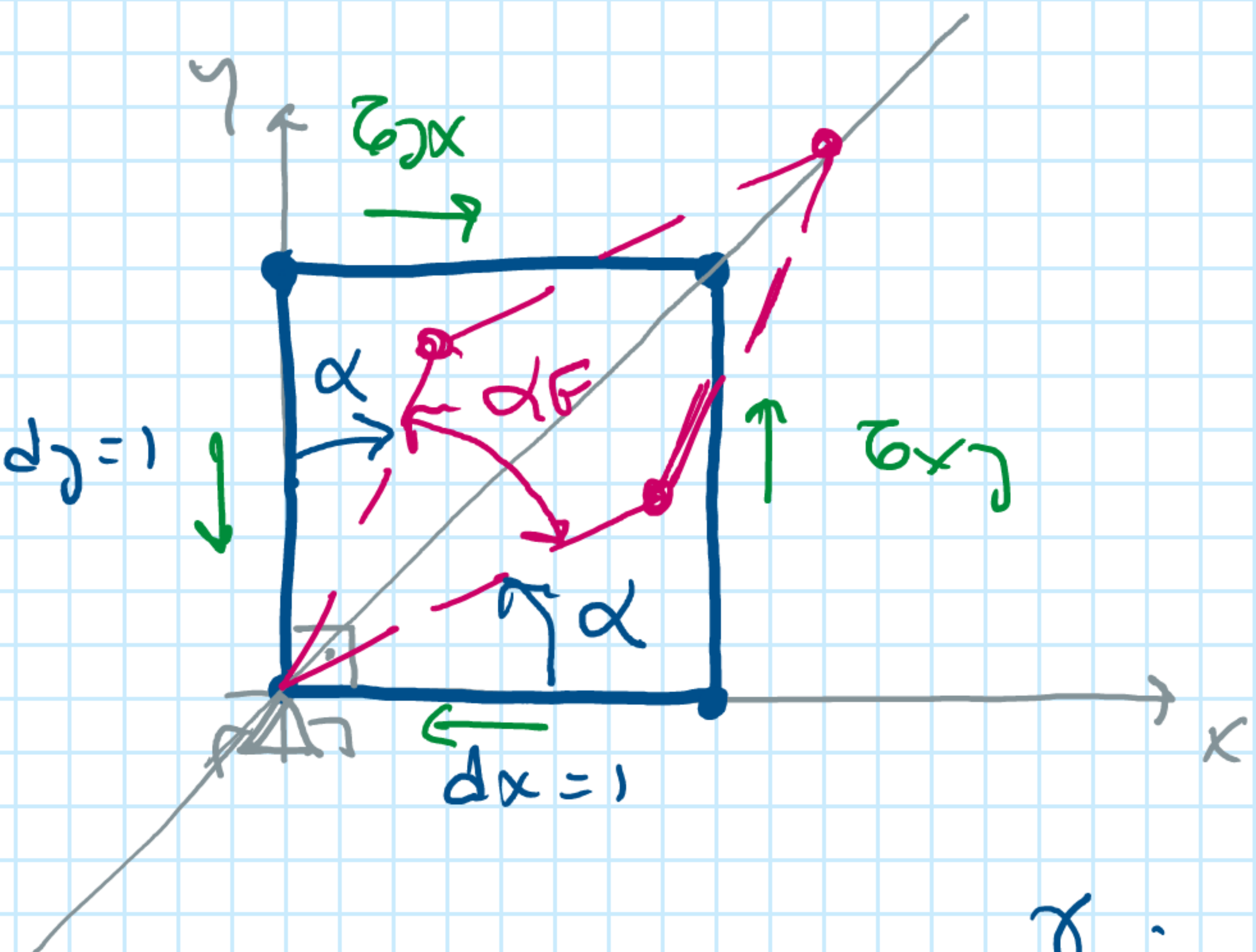
MISMO LO MISMO P/ LOS OTROS PLANOS:

$$\epsilon_{xz} = \epsilon_{zx} \quad \wedge \quad \epsilon_{yz} = \epsilon_{zy}$$

$$\boxed{\epsilon_{ij} = \epsilon_{ji}}$$

# 09.03 - ENSAYOS:

martes, 30 de noviembre de 2021 11:25



$$\left\{ \begin{aligned} \alpha_{0,xy} &= \frac{\pi}{2} = 90^\circ \\ \alpha_{F,xy} & \end{aligned} \right.$$

$$\alpha_{0,xy} - \alpha_{F,xy} = \frac{\pi}{2} - \alpha_{F,xy} = \gamma_{xy}$$

$\gamma$ : Distorsión angular entre 2 direcciones, en este caso entre  $x$  e  $y$ .

$$\gamma_{xy} = \alpha + \alpha = 2\alpha = 2\varepsilon_{xy} = 2\varepsilon_{yx}$$

$$\gamma_{xy} = 2\varepsilon_{xy} = 2\varepsilon_{yx} = \gamma_{yx}$$

$$\varepsilon_{xy} = \frac{\gamma_{xy}}{2} \qquad \varepsilon_{yx} = \frac{\gamma_{yx}}{2}$$

$$\gamma_{ij} = 2\varepsilon_{ij} \quad \wedge \quad \varepsilon_{ij} = \frac{\gamma_{ij}}{2}$$

Si se aplica un  $\sigma_{xy}$  o  $\tau_{xy}$  se obtiene:

$$\gamma_{xy} = \frac{\tau_{xy}}{G} = 2\varepsilon_{xy} \quad \longleftrightarrow \quad \varepsilon_{xy} = \frac{\tau_{xy}}{2G} = \frac{\gamma_{xy}}{2}$$

$$\gamma_{yz} = \frac{\tau_{yz}}{G} = 2\varepsilon_{yz} \quad \longleftrightarrow \quad \varepsilon_{yz} = \frac{\tau_{yz}}{2G} = \frac{\gamma_{yz}}{2}$$

$$\gamma_{xz} = \frac{\tau_{xz}}{G} = 2\varepsilon_{xz} \quad \longleftrightarrow \quad \varepsilon_{xz} = \frac{\tau_{xz}}{2G} = \frac{\gamma_{xz}}{2}$$

# 09.04 - LEY DE HOOKE GENERALIZADA:

martes, 30 de noviembre de 2021

11:35

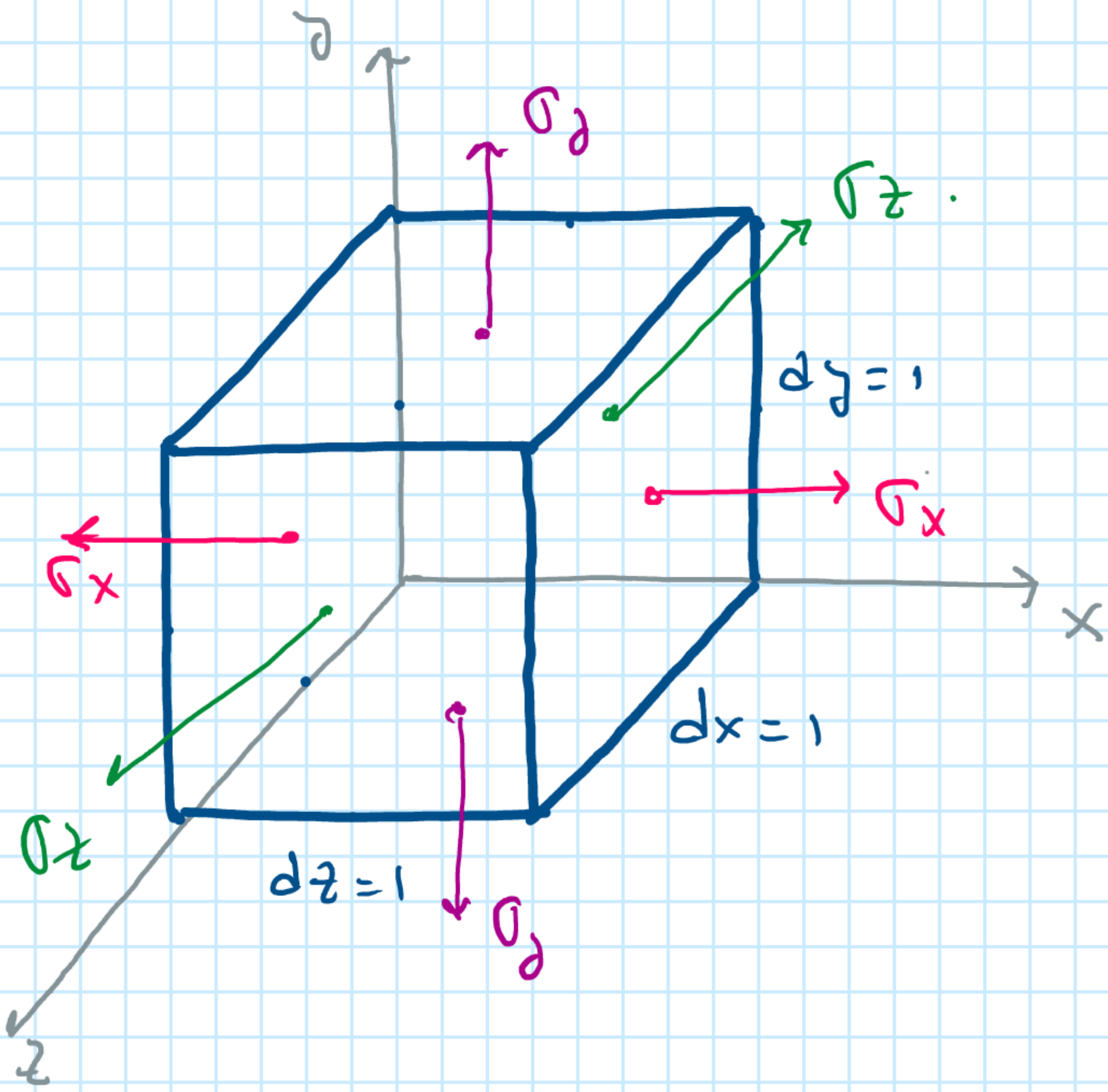
$$\begin{pmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{pmatrix} = \begin{bmatrix} 1/E & -\mu/E & -\mu/E & 0 & 0 & 0 \\ -\mu/E & 1/E & -\mu/E & 0 & 0 & 0 \\ -\mu/E & -\mu/E & 1/E & 0 & 0 & 0 \\ \dots & \dots & \dots & \dots & \dots & \dots \\ 0 & 0 & 0 & 1/2G & 0 & 0 \\ 0 & 1/2G & 0 & 0 & 1/2G & 0 \\ 0 & 0 & 1/2G & 0 & 0 & 1/2G \end{bmatrix} \begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{pmatrix}$$

$E$  : módulo de elasticidad longitudinal

$G$  : " " " transversal

$\mu$  : coeficiente de Poisson

CONSTANTES ELÁSTICAS  
DEL MATERIAL.



$$V_0 = dx \cdot dy \cdot dz = 1 \cdot 1 \cdot 1 = 1$$

**X**

$$\begin{cases} L_{fx} = L_{ox} + \Delta L_x = dx + \Delta L_x \\ L_{fx} = 1 + \Delta L_x \end{cases}$$

$$\Delta L_x = L_{fx} - L_{ox} = L_{fx} - 1$$

$$\Delta L_x = 1 + \underbrace{\Delta L_x}_{\Delta dx} - 1 = \Delta dx$$

$$\epsilon_{xx} = \frac{\Delta L_x}{L_{ox}} = \frac{\Delta dx}{L_{ox}} = \frac{\Delta dx}{dx} = \Delta dx$$

$$\underline{\epsilon_{xx} = \Delta dx}$$

**Y**  $\epsilon_{yy} = \Delta dy$

**Z**  $\epsilon_{zz} = \Delta dz$

$$V_f = L_{fx} \cdot L_{fy} \cdot L_{fz} = (1 + \Delta dx)(1 + \Delta dy)(1 + \Delta dz) =$$

$$= (1 + \epsilon_{xx})(1 + \epsilon_{yy})(1 + \epsilon_{zz}) =$$

$$= 1 + \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz} + \cancel{\epsilon_{xx}\epsilon_{yy}} + \cancel{\epsilon_{xx}\epsilon_{zz}} + \cancel{\epsilon_{yy}\epsilon_{zz}} + \cancel{\epsilon_{xx}\epsilon_{yy}\epsilon_{zz}}$$

2 ki potestas

TERMINOS DE O.S.

i) → de deformaciones sostenidas

ii) → " " deformaciones

$$V_f = 1 + \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$$

$$\Delta V = V_f - V_0 = 1 + \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz} - 1 \rightarrow$$

$$\rightarrow \Delta V = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}$$

$$\epsilon_V = \frac{\Delta V}{V_0} = \frac{\epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}}{1} \rightarrow$$

$$\epsilon_V = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz} = \text{traza}[\epsilon] = I_1[\epsilon]$$

$$\epsilon_V = \frac{1}{2} [\underbrace{\sigma_x - \mu(\sigma_y + \sigma_z)}] + \frac{1}{2} [\underbrace{\sigma_y - \mu(\sigma_x + \sigma_z)}] + \frac{1}{2} [\underbrace{\sigma_z - \mu(\sigma_x + \sigma_y)}]$$

$$\epsilon_V = \frac{1}{2} [(\sigma_x - 2\mu\sigma_x) + (\sigma_y - 2\mu\sigma_y) + (\sigma_z - 2\mu\sigma_z)]$$

$$\epsilon_V = \frac{1}{2} [\sigma_x(1-2\mu) + \sigma_y(1-2\mu) + \sigma_z(1-2\mu)]$$

$$\epsilon_V = \frac{(1-2\mu)}{2} (\sigma_x + \sigma_y + \sigma_z) = \frac{(1-2\mu)}{2} \text{traza}[\sigma] = \frac{(1-2\mu)}{2} I_1[\sigma]$$



$$\epsilon_V = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz} = \text{traza}[\epsilon] = I_1[\epsilon]$$

$$\epsilon_V = \frac{(1-2\mu)}{E} (\sigma_x + \sigma_y + \sigma_z) = \frac{(1-2\mu)}{E} \text{traza}[\sigma] = \frac{(1-2\mu)}{E} I_1[\sigma]$$

particularidades:

I  $\sigma_x = \sigma_y = \sigma_z = p > 0$

$$\epsilon_V = \frac{(1-2\mu)}{E} \cdot 3p = \frac{\Delta V}{V_0} > 0 \quad \text{Porque } \Delta V > 0$$

$$(1-2\mu) \frac{3p}{E} > 0 \rightarrow 1-2\mu \geq 0 \rightarrow \boxed{\mu < \frac{1}{2} = 0,5}$$

II  $\epsilon_V = \frac{3(1-2\mu)}{E} p = \frac{p}{k} \rightarrow \epsilon_V = \frac{p}{k}$

$p = k \cdot \epsilon_V$  ;  $k = \frac{E}{3(1-2\mu)}$

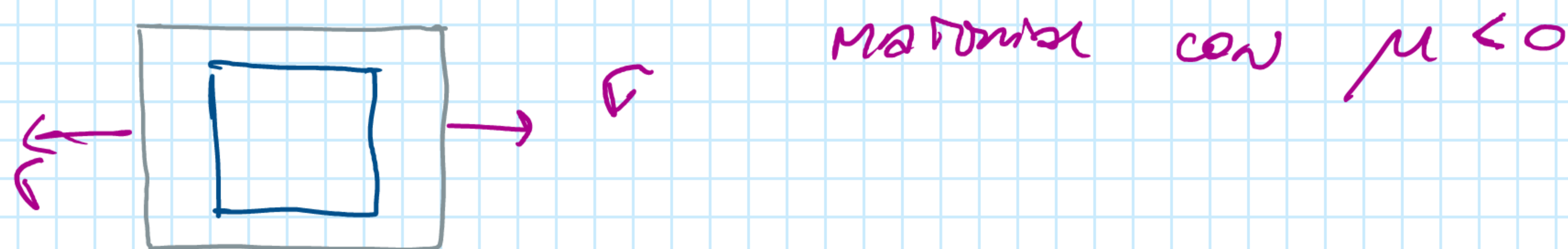
• si  $\mu \rightarrow 0,5 \rightarrow k \rightarrow \infty \rightarrow \boxed{\epsilon_V \rightarrow 0}$

$\mu \rightarrow 0,5 \rightarrow$  el material es casi incompresible.

si  $\mu = 0,5 \rightarrow$  " " seria infinitamente incompresible.

• si  $\mu \rightarrow 0 \rightarrow k = \frac{E}{3} \rightarrow \boxed{\epsilon_V = \frac{3p}{E}}$

• si  $\mu \rightarrow -\infty \rightarrow k \rightarrow 0 \rightarrow \epsilon_V \rightarrow \infty \rightarrow$  rigidez nula.



$0 \leq \mu < 0,5$

•  $\rightarrow$  aumenta la rigidez

•  $\leftarrow$  disminuye la rigidez.

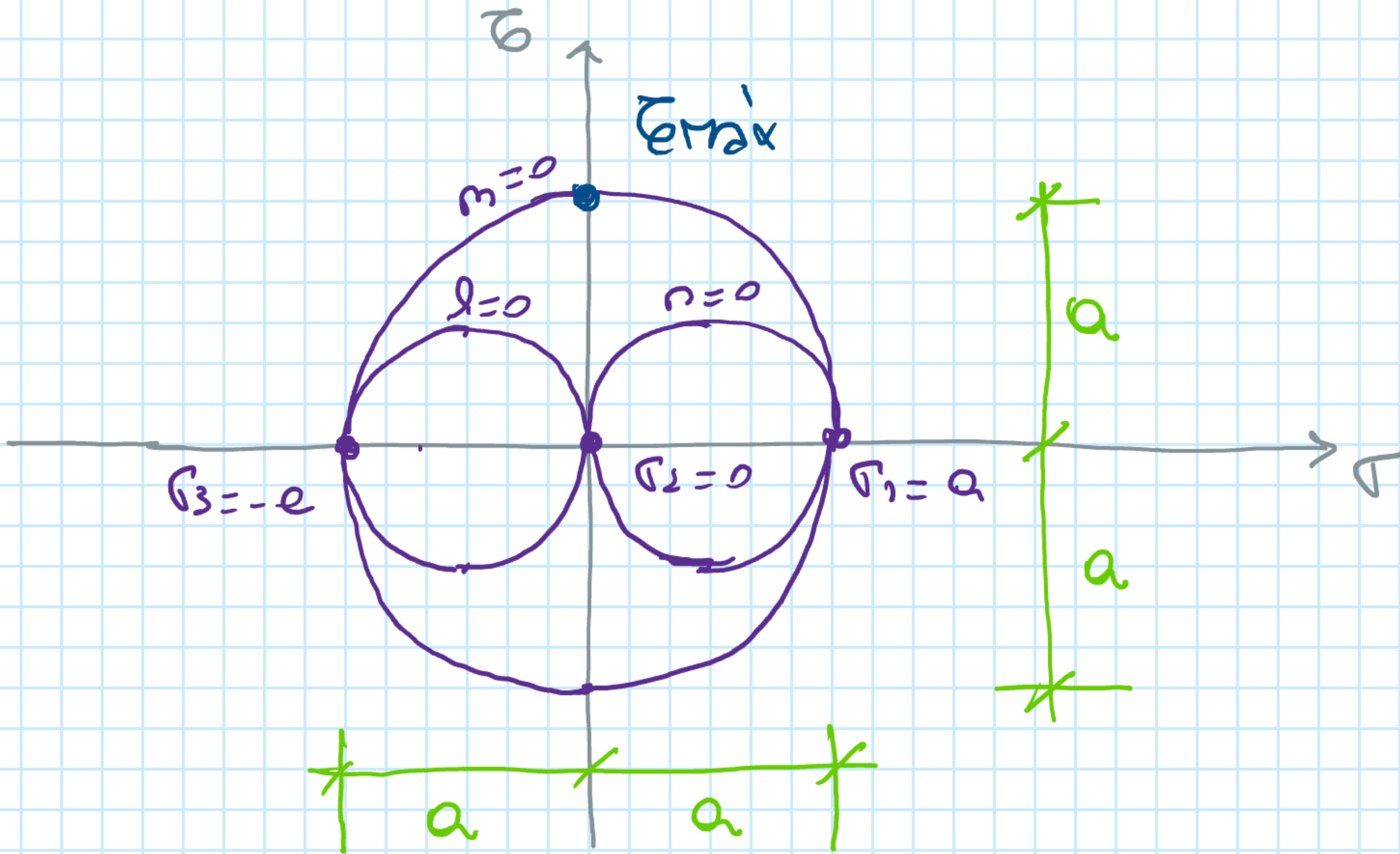
• si  $\mu > 0,5 \rightarrow k < 0$  un material traccional  $\rightarrow$  disminuye su volumen

VAMOS A TRABAJAR CON UN CASO PARTICULAR DE TENSIONES:

→ FORMA PRINCIPAL:

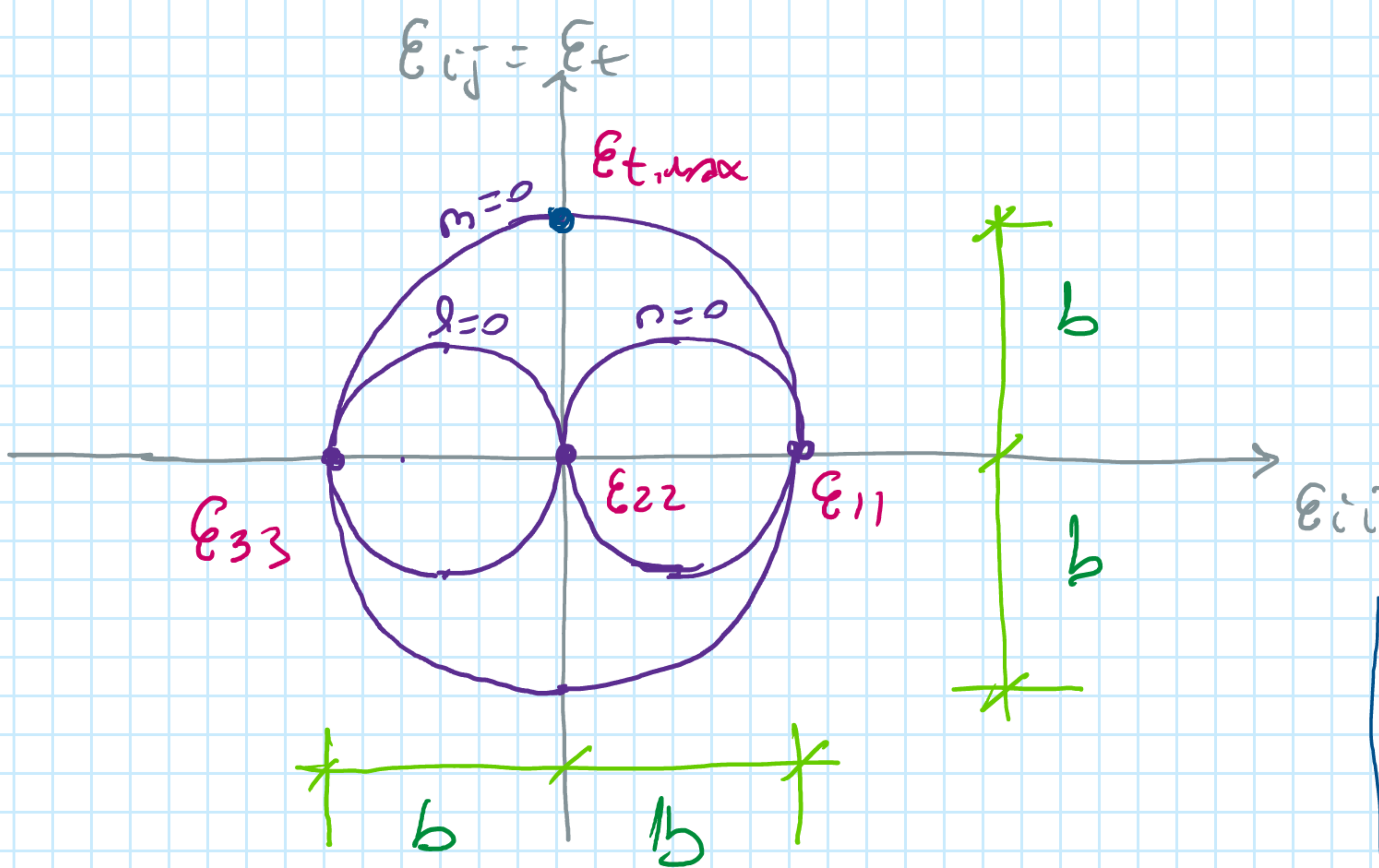
$$\begin{cases} \sigma_1 = a \\ \sigma_2 = 0 \\ \sigma_3 = -a \end{cases}$$

• MOSTRAMEMOS A TRAVÉS DE UN CIRCULO DE MOHR:



$$\begin{cases} \epsilon_{11} = \frac{1}{E} [\sigma_1 - \mu(\sigma_2 + \sigma_3)] = \frac{1}{E} [a - \mu(-a)] = \frac{(1+\mu)}{E} a \\ \epsilon_{22} = \frac{1}{E} [\sigma_2 - \mu(\sigma_1 + \sigma_3)] = \frac{1}{E} [0 - \mu(a-a)] = 0 \\ \epsilon_{33} = \frac{1}{E} [\sigma_3 - \mu(\sigma_1 + \sigma_2)] = \frac{1}{E} [-a - \mu(a+0)] = -\frac{(1+\mu)}{E} a \end{cases}$$

$\epsilon_{t, \max} = ?$   $\left[ \epsilon_{t, \max} = \frac{\tau_{\max}}{2G} = \frac{a}{2G} = b \right]$



$\epsilon_{11} = \epsilon_{t, \max}$   
 $\frac{(1+\mu)}{E} \cdot a = \frac{a}{2G}$

$$\begin{aligned} G &= \frac{E}{2(1+\mu)} \\ E &= 2(1+\mu)G \\ \mu &= \frac{E}{2G} - 1 \end{aligned}$$

$\epsilon_{ij} = \frac{\tau_{ij}}{2G} = \frac{\tau_{ij}}{2 \cdot \frac{E}{2(1+\mu)}} \rightarrow \epsilon_{ij} = \frac{1+\mu}{E} \tau_{ij} \leftarrow$

# 09.07 - LEY DE HOOKE GENERALIZADA:

martes, 30 de noviembre de 2021 12:37

$$\begin{pmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{pmatrix} = \frac{1}{E} \begin{pmatrix} 1 & -\mu & -\mu & 0 & 0 & 0 \\ -\mu & 1 & -\mu & 0 & 0 & 0 \\ -\mu & -\mu & 1 & 0 & 0 & 0 \\ \dots & \dots & \dots & \dots & \dots & \dots \\ 0 & 0 & 0 & (1+\mu) & 0 & 0 \\ 0 & 0 & 0 & 0 & (1+\mu) & 0 \\ 0 & 0 & 0 & 0 & 0 & (1+\mu) \end{pmatrix} \begin{pmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \\ \tau_{xy} \\ \tau_{yz} \\ \tau_{zx} \end{pmatrix}$$

$$\epsilon_{ij} = \frac{\sigma_{ij}}{E} = \frac{(1+\mu)}{E} \tau_{ij} \rightarrow \sigma_{ij} = \frac{2(1+\mu)}{E} \tau_{ij}$$

$$\sigma_{ij} = \frac{\tau_{ij}}{G}$$