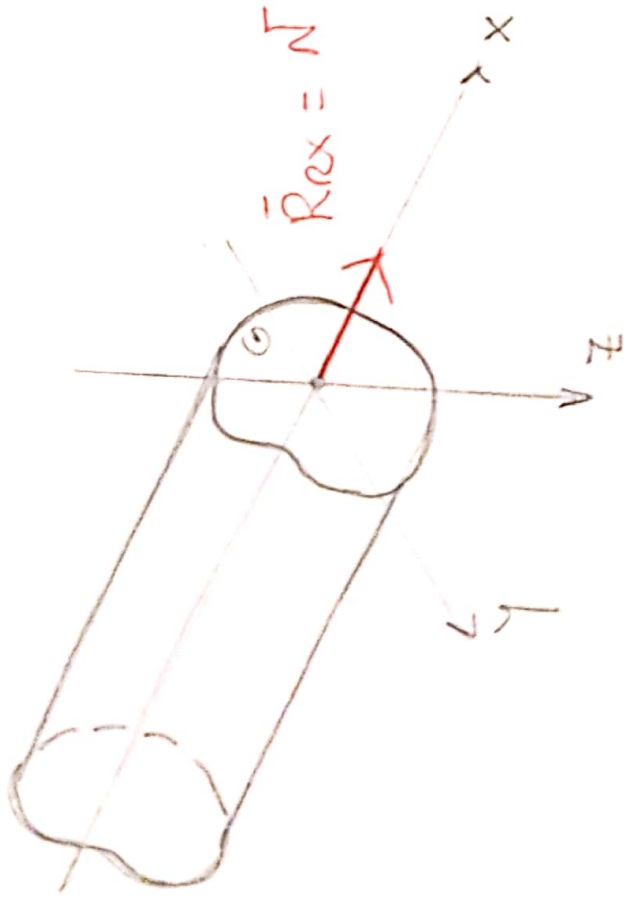


10/25

SOLICITACION AXIAL:

1) - OBJETO:

2) - DEFINICION:



3) - EC. DE EQUIVALENCIA:

$$1) \quad N = \int_A \sigma_x \cdot dA$$

$$2) \quad Q_y = 0 = \int_A \sigma_{xy} \cdot dA$$

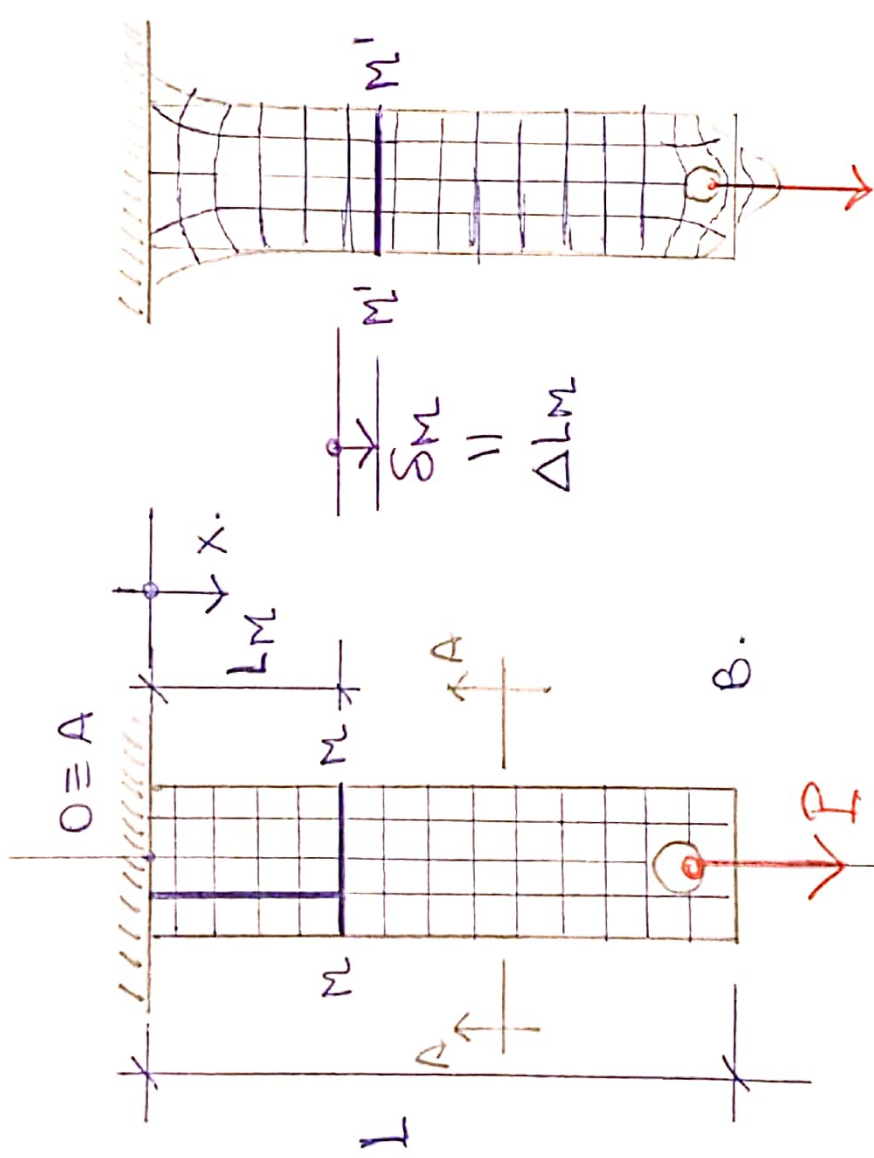
$$3) \quad Q_z = 0 = \int_A \sigma_{xz} \cdot dA$$

$$4) \quad M_x = 0 = \int_A (-\sigma_{xy} \cdot z + \sigma_{xz} \cdot y) dA$$

$$5) \quad M_y = 0 = \int_A \sigma_x \cdot z \cdot dA$$

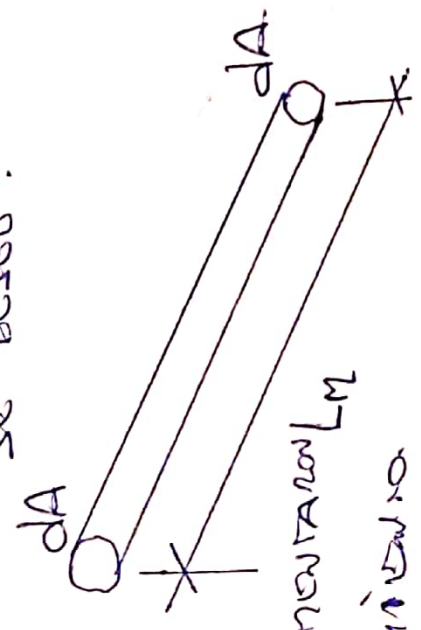
$$6) \quad M_z = 0 = \int_A (-\sigma_x) \cdot y \cdot dA$$

4) - EXPLICACIÓN DEL FENÓMENO:



• LAS SECCIONES SE MANTIENEN PLANAS Y PARALELAS A SÍ MISMAS.

• FIBRAS: ES UN PRISMA DE SECCIÓN INFINITESIMA, PARALELA AL EJE DE LA BARRA. > DE LA LONGITUD QUE SE DESEA.



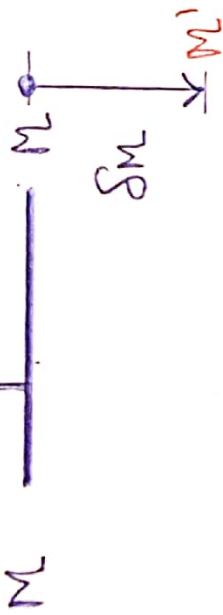
LAS FIBRAS EXPERIMENTAN EL MISMO A LARGAMIENTO ΔL_M



CORTE A-A

12/25

FIBRA
INICIAL



FIBRA FINAL



FIBRA: $L_0 = L_M \rightarrow \Delta L_M \rightarrow \Delta L = CTE$ P/ TODAS LAS FIBRAS Y P/ROS DE LA SECCIÓN.

$$\frac{\Delta L}{L_0} = \underbrace{\epsilon_x}_{\text{DEFORMACIÓN ESPECÍFICA LONGITUDINAL}} = CTE$$

$$\sigma_x = \underbrace{E \cdot \epsilon_x}_{CTE} = \boxed{CTES}$$

EN [SA] P/ SECCIONES ALEJADAS DE LA BARRA DE APLICACIÓN DE CARGAS Y DE APOYOS; PARA TODOS LOS PUNTOS DE LA MISMA. SE COMPROBABA QUE: $\epsilon_x = CTE$ y P/ LAS FIBRAS QUE $\Delta L = CTE$.

$$\sigma_x = CTE$$

DE LA EC. DE EQUIVALENCIA (1):

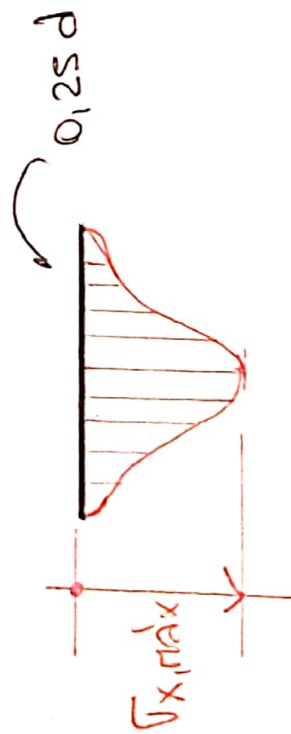
$$\sigma = \int_A \underbrace{\sigma_x}_{\text{cte}} \cdot dA = \sigma_x \int_A \frac{dA}{A}$$

$$\sigma_x(x) = \frac{N(x)}{A(x)}$$

→

$$\sigma_x \cdot A \cdot \rightarrow$$

→ HIPÓTESIS DE TRAZADO → NAVIER → "HIP. DE NAVIER"

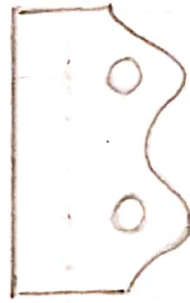
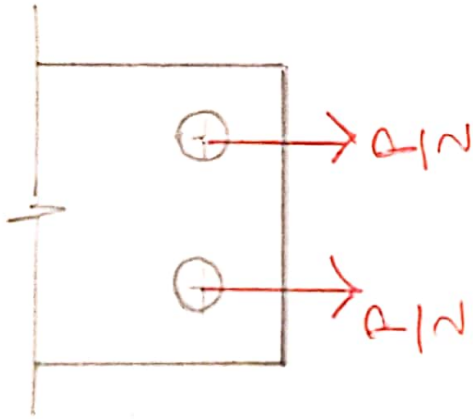


d



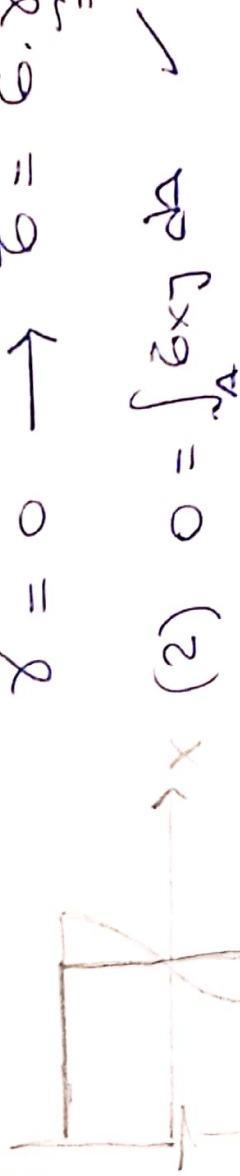
$$\sigma_{x, \text{MÁX}} = 2,60 \sigma_{x, \text{MEDIA}}$$

• si las secciones se mantienen planas:



↳ **I** → las secciones NO se alabean

$$\gamma = 0 \rightarrow \tau = 0 \stackrel{=0}{=} 0$$

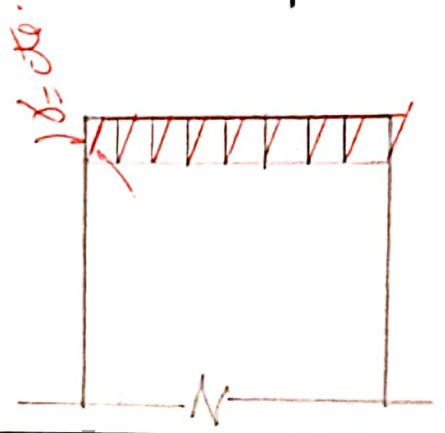


(2) $0 = \int_A \tau_{xy} dA$ ✓

(3) $0 = \int_A \tau_{xz} dA$ ✓

↳ **II** →

las secciones ~~se~~ NO se alabean pero → $\gamma = cte.$



$$\rightarrow \tau = 0 \cdot \gamma = cte.$$

$$\begin{aligned}
 (2) \quad 0 &= \int_A \bar{\sigma}_{xy} \, dA = \bar{\sigma}_{xy} \int_A dA. \rightarrow 0 = \int_A dA \rightarrow \\
 (3) \quad 0 &= \int_A \bar{\sigma}_{xz} \, dA = \bar{\sigma}_{xz} \int_A dA. \rightarrow 0 = \int_A dA. \rightarrow
 \end{aligned}$$

→ 1º) inconmensura matemática → 2º) unidades físicas. →

$\bar{\sigma}_{xy} = \bar{\sigma}_{xz} = 0$

• LA SECCIÓN NO SE AUSAZA 3 NO SE DISPONSIOMA.

$$(4) \Rightarrow M_x = 0 = \int_A (-\bar{\sigma}_{xy} \cdot z + \underbrace{\bar{\sigma}_{xz} \cdot y}_{=0}) \, dA \quad \checkmark$$

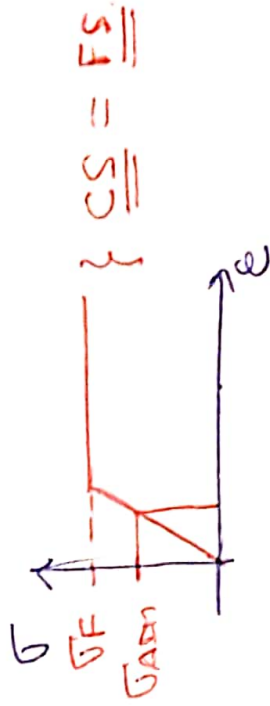
$$(5) \quad M_y = 0 = \int_A \sigma_x \cdot z \, dA = \cancel{\sigma_x} \int_A z \, dA. = \int_A z \, dA \quad \checkmark$$

$$M_z = 0 = \int_A (-\sigma_x) \cdot y \, dA = (-\cancel{\sigma_x}) \int_A y \, dA = \int_A y \, dA \quad \checkmark$$

MOMENTOS ESTÁTICOS
RESPON ESTÁTICOS de BANDA CON.

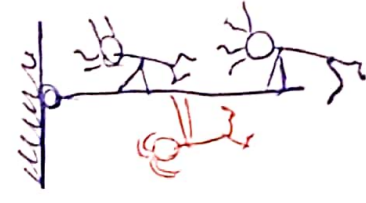
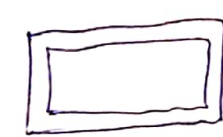
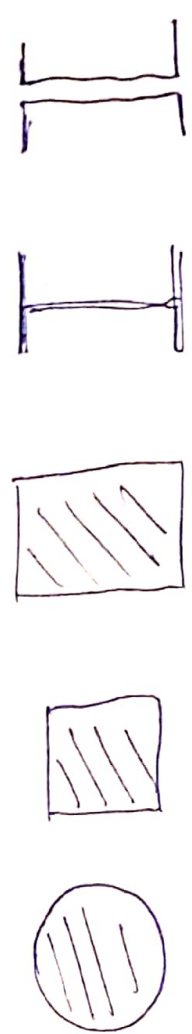
$\sigma_x(x) = \frac{N(x)}{A(x)}$ → VERIFICAR LOS ELEMENTOS ESTRUCTURALES.

$$\sigma_x \leq \sigma_{x,ADM} = \frac{\sigma_F}{CS}$$



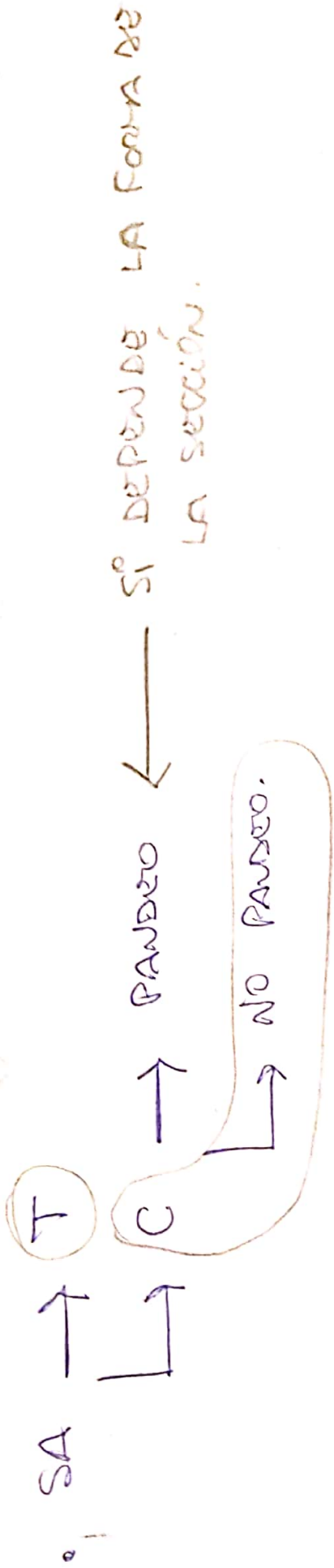
$A(x) \Rightarrow \frac{N(x)}{\sigma_{ADM}}$

→ DIMENSIONADO O DIMENSIONAMIENTO.



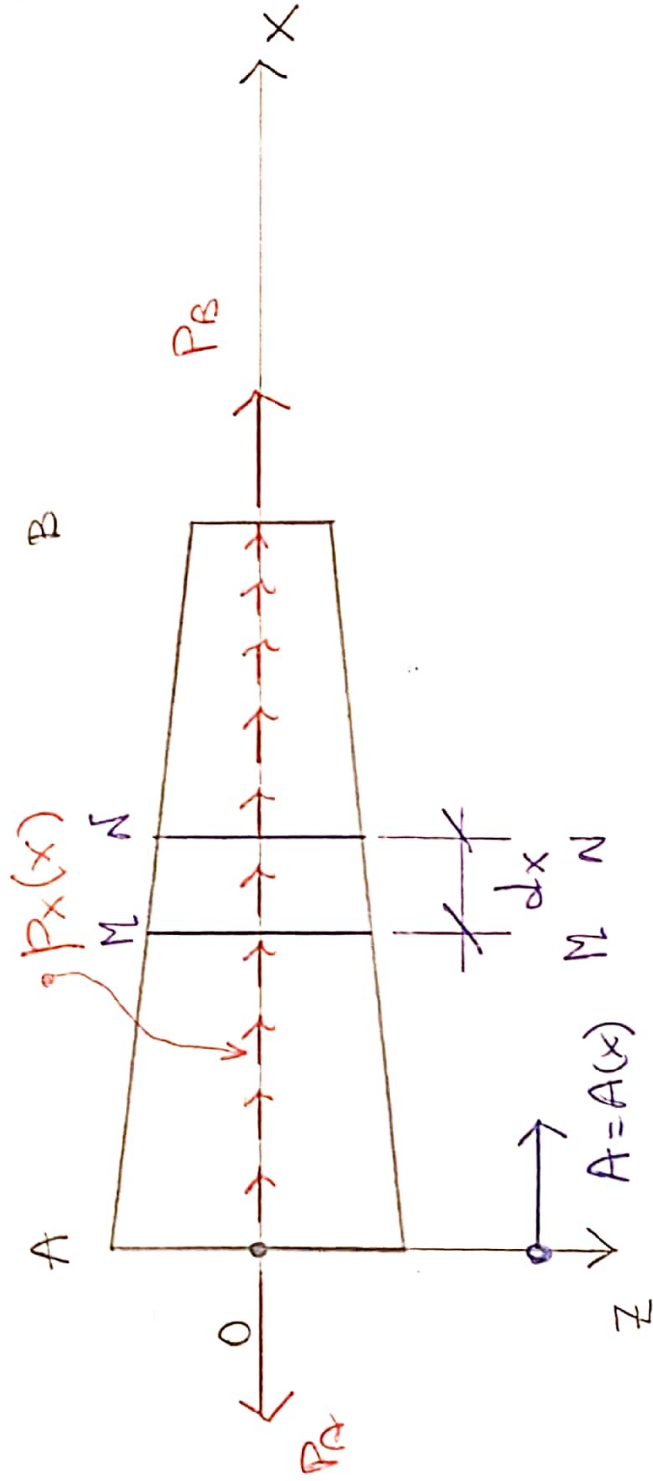
$N(x) = \sigma_x \cdot A(x)$ → IDENTIFICACIÓN.

$$N_{ADM} = \sigma_{ADM} \cdot A(x)$$

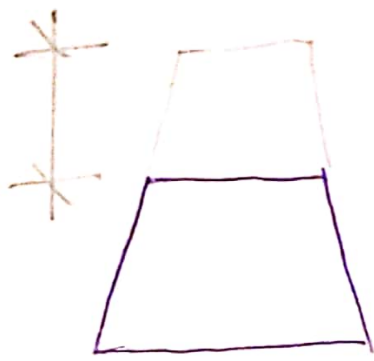
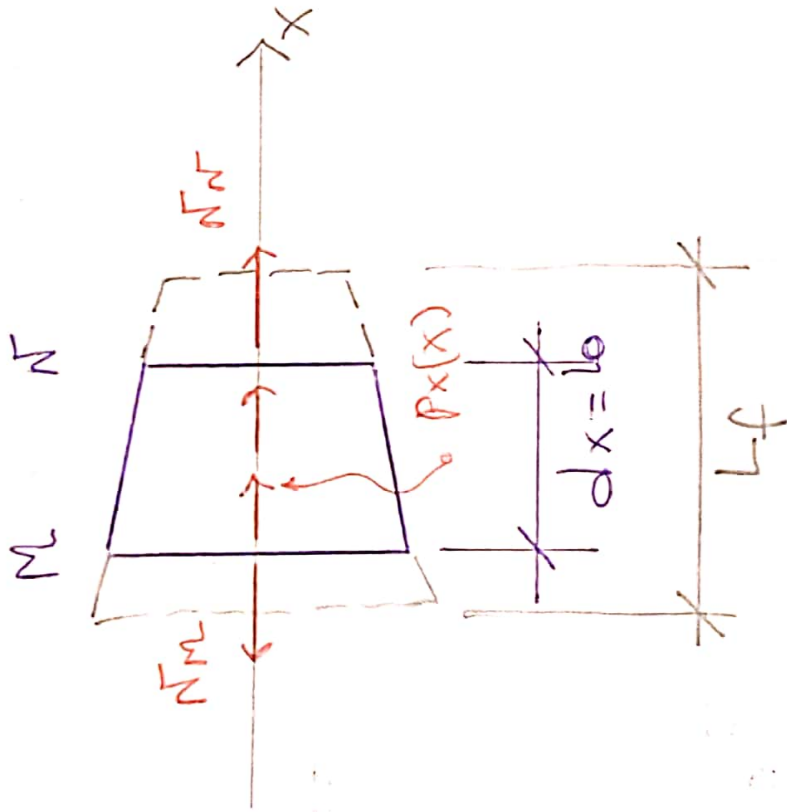


105) → DESPLAZAMIENTOS Y DEFORMACIONES:

REBANADA
ELEMENTAL.
 $L \rightarrow dx$



$$\Delta L = \Delta dx.$$



$$\frac{\Delta dx}{dx} = \epsilon_x(x)$$

$$\Delta dx = \epsilon_x(x) \cdot dx.$$

• Ley de Hooke

$$\sigma_x(x) = E \cdot \epsilon(x).$$

$$\Delta dx = \frac{\sigma_x(x)}{E} \cdot dx.$$

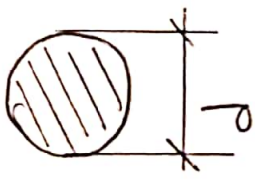
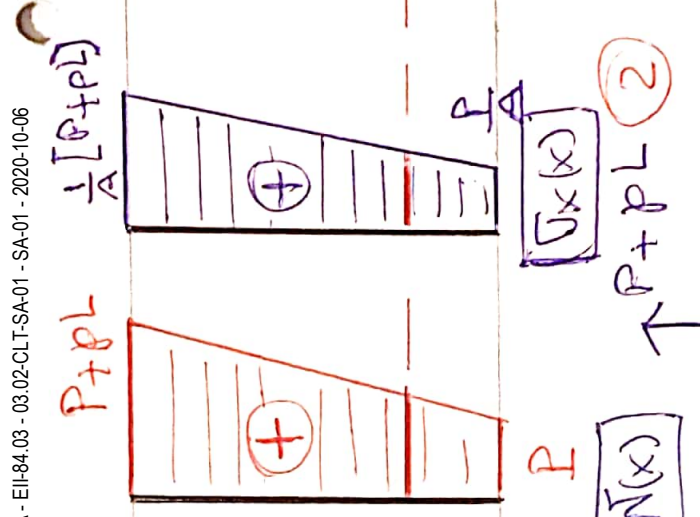
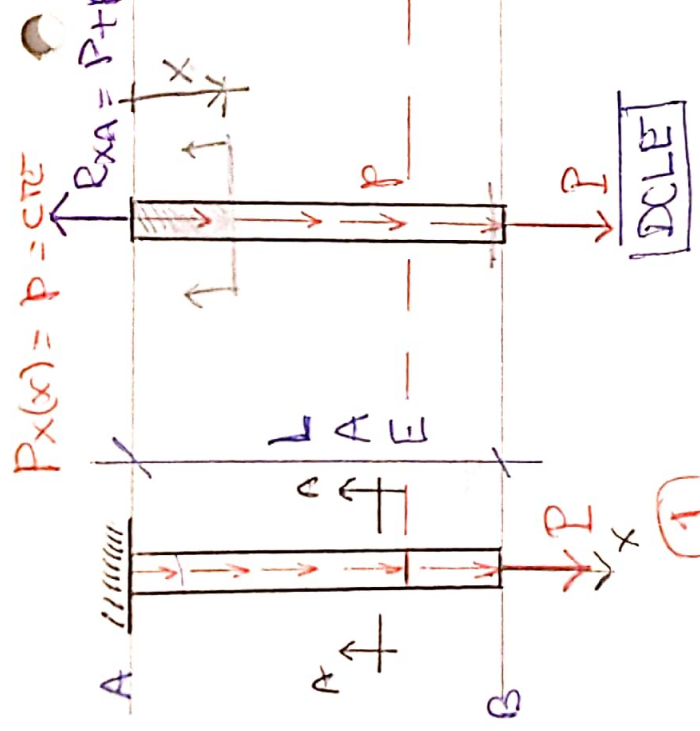
• si $\sigma_x(x) = \frac{N(x)}{A(x)}$

$$\Delta L = L_f - l_0 = \Delta dx$$

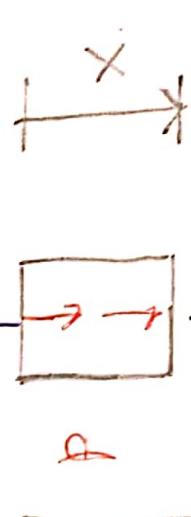
$$\Delta dx = \frac{N(x)}{EA(x)} \cdot dx.$$

$$\Delta L(x) = \int \Delta dx = \int \frac{N(x)}{EA(x)} dx.$$

$$\frac{d\Delta L(x)}{dx} = \frac{dN(x)}{dx} = \epsilon_x(x)$$



① $\sum F_x = 0$
 $P + p \cdot L + R_{xA} = 0$
 $R_{xA} = -P - pL$



$N(x) + P \cdot x - (P + pL) = 0$
 $N(x) = P + pL - px$
 $N(x) = P + p(L-x)$

$x=0 \quad N(x=0) = P + pL$
 $x=L \quad N(x=L) = P$

③ $V_x(x) = \frac{d(x)}{A(x)} = \frac{d(x)}{A}$
 $V_x(x) = \frac{1}{A} [P + p(L-x)]$

$$\delta(x) = \int \frac{N(x)}{EA(x)} dx = \frac{1}{EA} \int N(x) dx$$

$$\delta(x) = \frac{1}{EA} \int [P + P(L-x)] dx =$$

$$= \left[\frac{1}{EA} \left[Px + PLx - \frac{1}{2}Px^2 \right] \right]$$

$$\delta(x) = \frac{1}{EA} \left[Px + P(Lx - \frac{x^2}{2}) \right]$$

$$\delta(x=0) = \frac{1}{EA} \left[P \cdot 0 + P(L \cdot 0 - \frac{0^2}{2}) \right] = 0$$

$$\delta(x=L) = \frac{1}{EA} \left[PL + PL^2 - P \frac{L^2}{2} \right]$$

$$\delta(x=L) = \frac{1}{EA} \left[PL + \frac{1}{2}PL^2 \right]$$

Autor: Frig. Tujó Nelson SOSTI



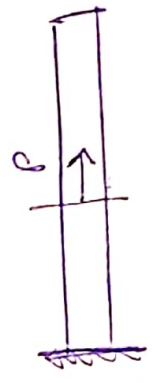
$$\frac{d\delta(x)}{dx} = \frac{1}{EA} [P + PL - Px]$$

$x \rightarrow L \rightarrow$ LA PENDIENTE DIMINUYE.

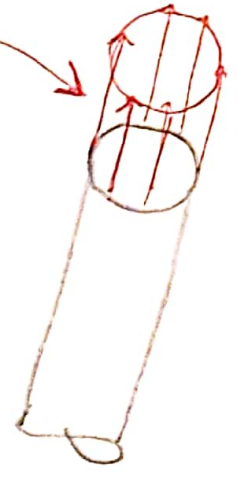
$$\epsilon_x(x) = \frac{1}{EA} [P + PL - Px]$$

$$\epsilon_x(x=0) = \frac{1}{EA} [P + PL]$$

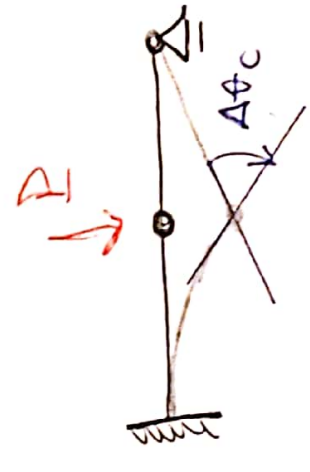
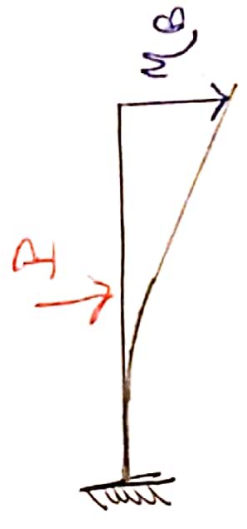
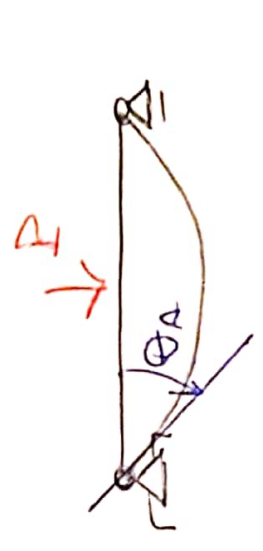
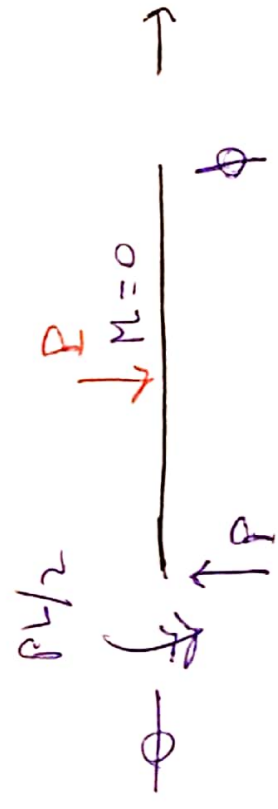
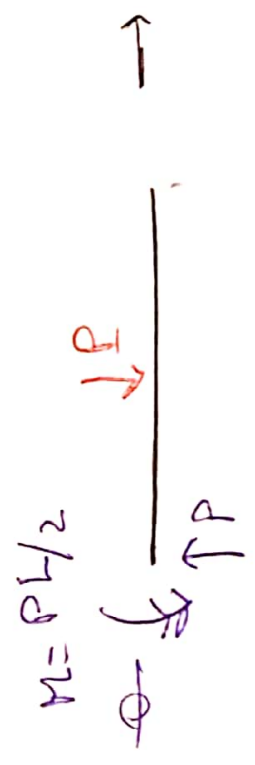
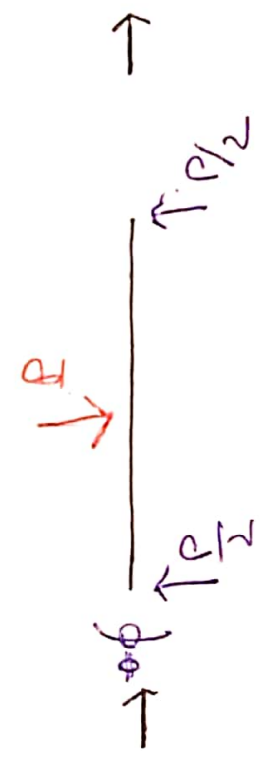
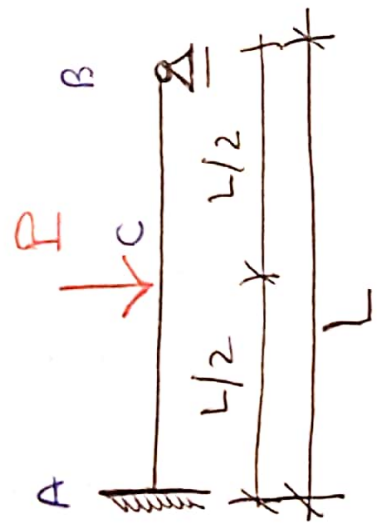
$$\epsilon_x(x=0) = \frac{P}{EA}$$



VOLUMEN DE VERSIONES



Método de Incompatibilidades Estáticas / Resolución de sistemas hiperestáticos:



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FIUBA - EIH-84.03 - 03.02-CLT-SA-01 - SA-01 - 2020-10-06

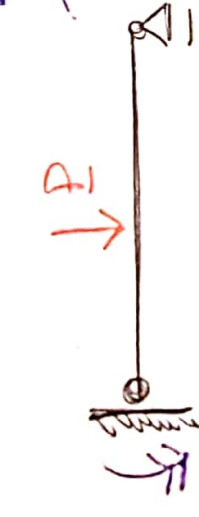
PRV
PARC.

$e_{ci} = k_{ci} \quad e_i = +1$

SF: SISTEMA FUNDAMENTAL O SIST. CERO.



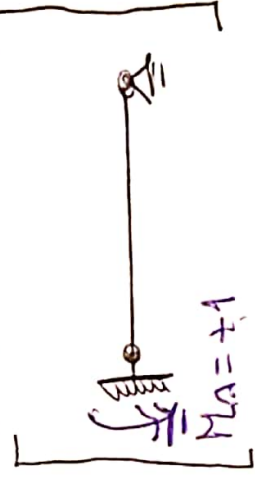
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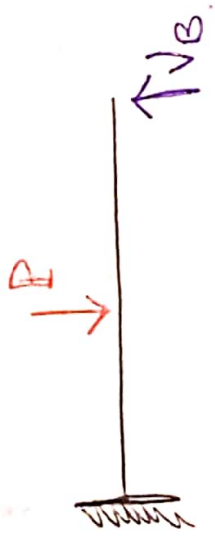
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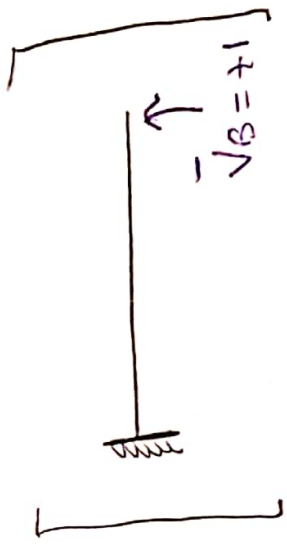
Ma.



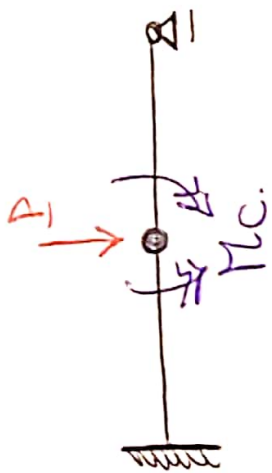
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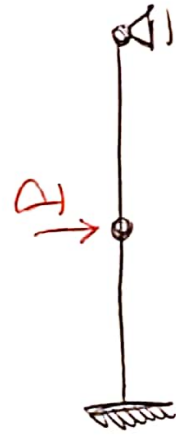
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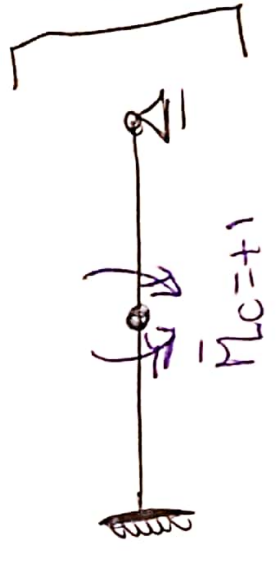
Vb.



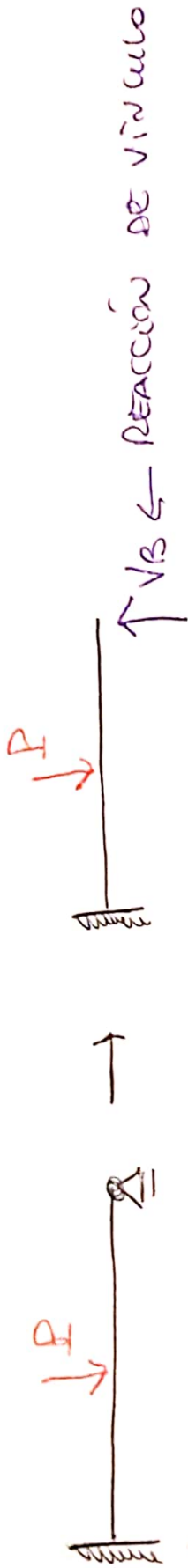
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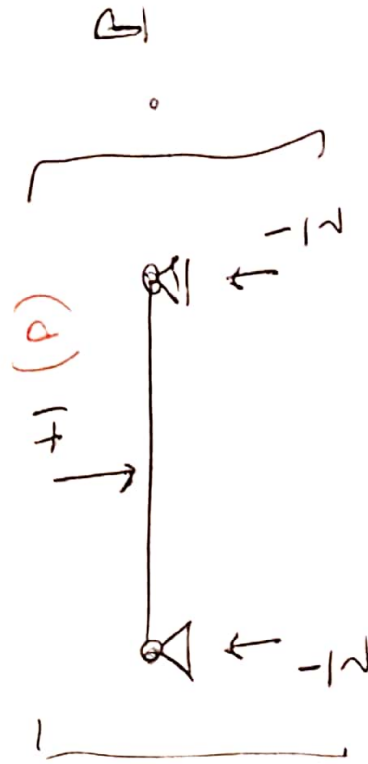
Mc.



$\downarrow V_B \leftarrow$ ACCIÓN DE LA ESTRUCTURA SOBRE EL VÍNCULO.

PRV

PAYR



$\frac{P}{2}$

$\frac{P}{2}$

PRV:

PRINCIPIO DE LAS REACCIONES VINCULARES

PAYR

PRINCIPIO DE ACCIÓN Y REACCIÓN.

PSE: PRINCIPIO DE SUPERPOSICIÓN DE EFECTOS

24/25

$$\theta_{A,CF}^H = 0 = \theta_{A,CF}^0 + \theta_{A,MA}^0 : MA$$

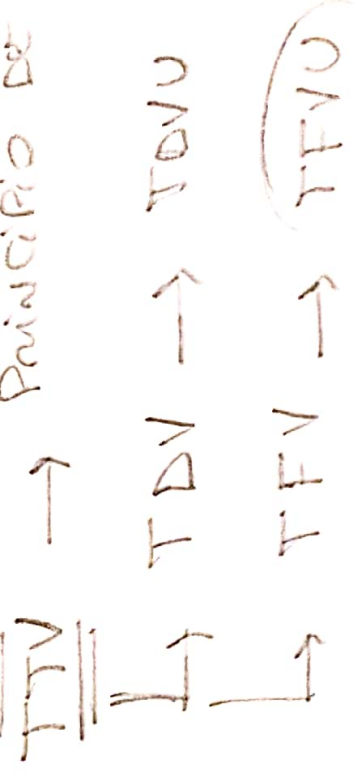
$$M_{B,CF}^H = 0 = M_{B,CF}^0 + M_{B,VB}^0 : VB$$

$$\Delta \theta_{C,CF}^H = 0 = \Delta \theta_{C,CF}^0 + \Delta \theta_{C,MC}^0 : MC$$

ECS. DE COMPATIBILIDAD DE LOS DESPLAZAMIENTOS.

→ INCÓGNITAS → ESTÁTICAS

LOS DESPLAZ → PRINCIPIO DE EQUIVALENCIA.

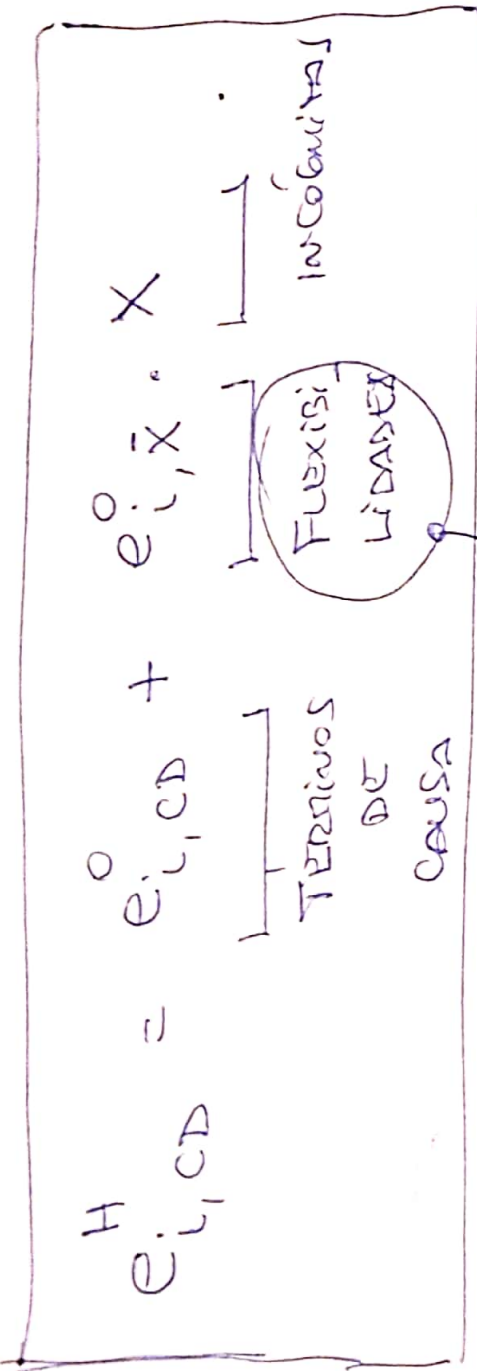


H: HIPERESTÁTICO
 O: SIST. FUNDAMENTAL
 CF: CAUSA FUERZAS

ΔT: VARIACIÓN DE TEMPERATURA → q
 ΔV: CEMENTO DE VÍNCULO

25/25
EII

$$GH = 1,$$



FLEXIBILIDADES

