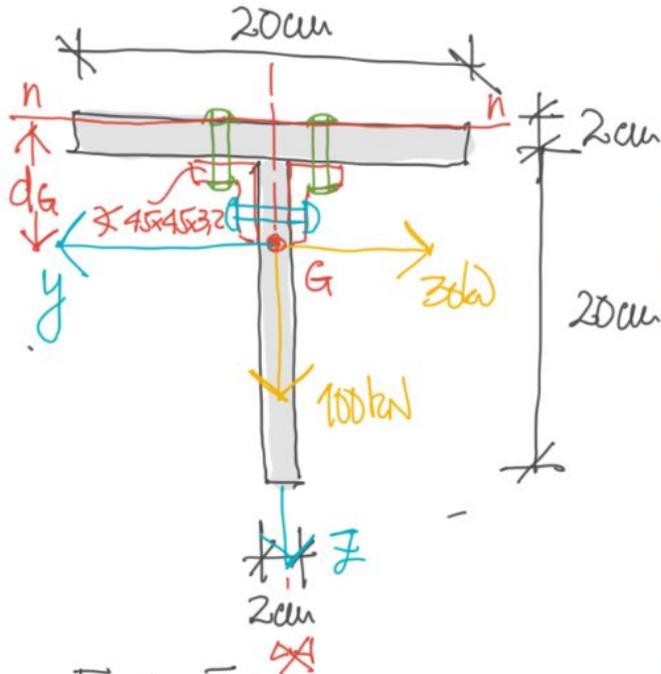


Ejercicio 4



$$F_b \geq F_{resb}$$

$$\tau_{admis} \cdot A_{bulón} \cdot n \geq \tau_{resb} \cdot b \cdot sep$$

$$\tau_{admis} \cdot A_{bulón} \cdot n \geq \frac{\phi \cdot S_{lw}^*}{J_{lw} \cdot b} \cdot b \cdot sep$$

$\tau_{bulón} = 10 \text{ kN/cm}^2$
 $\tau_{acero} = 8 \text{ kN/cm}^2$
 $sep = 20 \text{ cm}$
 Perfiles ángulo 45x45x 3,2 mm
 $A = 2,83 \text{ cm}^2$
 $J_z = J_y = 5,24 \text{ cm}^4$

$$S_{nn} = A_{tot} \cdot d_G = (20 \times 2) \text{ cm}^2 \times 1 \text{ cm} + 2 \times 2,83 \text{ cm}^2 \times (2 \text{ cm} + 1,19 \text{ cm}) + (20 \times 2) \text{ cm}^2 \times 12 \text{ cm}$$

$$85,16 \text{ cm}^2 \cdot d_G = 538,055 \text{ cm}^3 \rightarrow d_G = 6,28 \text{ cm}$$

$$J_y = \frac{20 \times 2^3}{12} \text{ cm}^4 + 40 \text{ cm}^2 \times (6,28 - 1)^2 \text{ cm}^2 + 2 \times 5,24 \text{ cm}^4 + 2 \times 2,83 \text{ cm}^2 \times (6,28 - 2 - 1,19)^2 \text{ cm}^2 + \frac{2 \times 20^3}{12} \text{ cm}^4 + 40 \text{ cm}^2 \times (12 - 6,28)^2 \text{ cm}^2$$

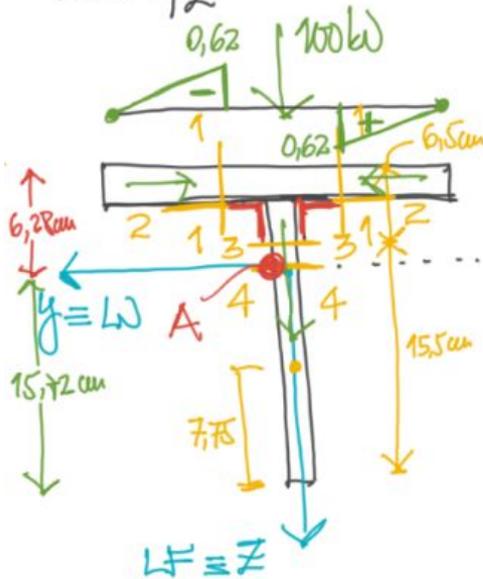
$$J_y = 3835,06 \text{ cm}^4$$

$$J_z = \frac{2 \times 20^3}{12} \text{ cm}^4 + \frac{20 \times 2^3}{12} \text{ cm}^4 + 2 \times 5,24 \text{ cm}^4 + 2 \times 2,83 \text{ cm}^2 \times (1,19 + 1)^2 \text{ cm}^2$$

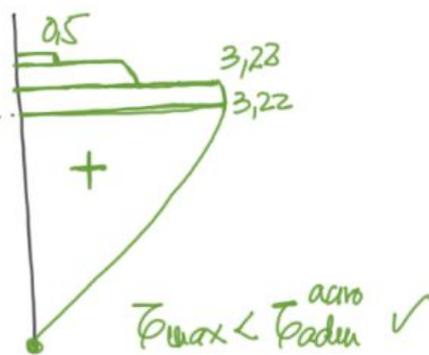
$$J_z = 1384,3 \text{ cm}^4$$

Calculamos los diagramas de τ

Para Q_z



$$\begin{aligned} \tau_{11} &= 0,62 \text{ kN/cm}^2 \\ \tau_{22} &= 0,5 \text{ kN/cm}^2 \\ \tau_{33} &= 3,22 \text{ kN/cm}^2 \\ \tau_{44} &= 3,22 \text{ kN/cm}^2 = \tau_{\max} \end{aligned}$$

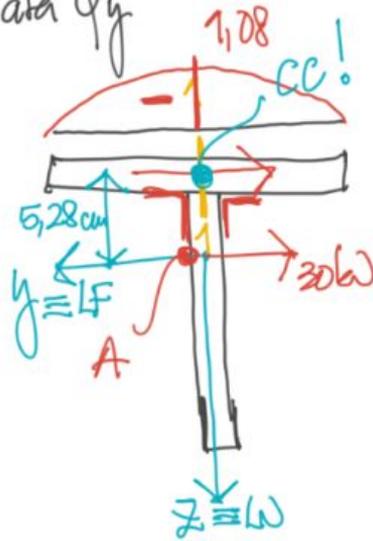


$\tau_{\max} < \tau_{\text{admis}} \checkmark$

$$\tau_{\text{resb}} = \frac{Q_z}{J_y} \frac{S_{y^*}}{b}$$

$$\begin{aligned} S_{y^*11} &= 4,5 \text{ cm} \times 2 \text{ cm} \times 5,28 \text{ cm} = 47,52 \text{ cm}^3 \\ S_{y^*22} &= 20 \text{ cm} \times 2 \text{ cm} \times 5,28 \text{ cm} = 211,2 \text{ cm}^3 \\ S_{y^*33} &= 15,5 \text{ cm} \times 2 \text{ cm} \times (20 \text{ cm} - 7,75 \text{ cm} - 6,28 \text{ cm}) = 247,07 \text{ cm}^3 \\ S_{y^*44} &= 15,72 \text{ cm} \times 2 \text{ cm} \times \frac{15,72 \text{ cm}}{2} = 247,12 \text{ cm}^3 \end{aligned}$$

Para Q_y



$$\tau_{\text{resb}} = \frac{Q_y}{J_z} \frac{S_{z^*}}{b}$$

$$\begin{aligned} S_{z^*11} &= 10 \text{ cm} \times 2 \text{ cm} \times 5 \text{ cm} = 100 \text{ cm}^3 \\ \tau_{11} &= \frac{30 \text{ kN} \times 100 \text{ cm}^3}{1384,3 \text{ cm}^4 \times 2 \text{ cm}} = 1,08 \text{ kN/cm}^2 \end{aligned}$$

$\tau_{11} = 30 \text{ kN} \times 5,28 \text{ cm}$
OJO!

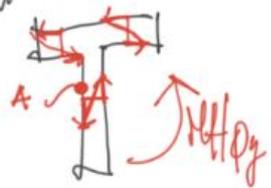
$$I_{yy} = 1584 \text{ cm}^4$$

$$J = \frac{1}{3} \times 2 \times (2 \text{ cm})^3 \times 20 \text{ cm}$$

$$J = 106,67 \text{ cm}^4$$

$$\tau_{\text{adm}} = \frac{\tau}{\tau} \cdot \tau$$

$$\tau_{\text{adm}} = 2,97 \text{ kN/cm}^2$$

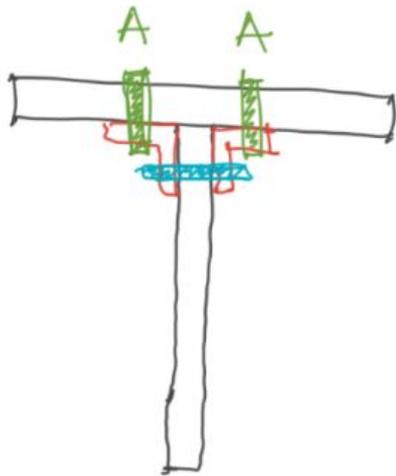


$$\tau_{\max} = \tau_A = 2,97 + 3,22$$

$$\tau_{\max} = 6,19 \text{ kN/cm}^2 < \tau_{\text{admis}} \text{ (Acero } 8 \text{ kN/cm}^2)$$

CUMPLE

Calculo los bulones



$$\underbrace{\tau_{adm}}_{10 \text{ kg/cm}^2} \times A_{bulm} \times 2 \geq 9,5 \text{ kg/cm}^2 \times \underbrace{11 \text{ cm} \times 20 \text{ cm}}_{\substack{\text{Este fue el "b"} \\ \text{que usamos p/ calc.} \\ \tau_{22/07}}$$

$$\textcircled{1} A_{bulm} = 5,5 \text{ cm}^2$$

$$\underbrace{\tau_{adm}}_{10 \text{ kg/cm}^2} \times A_{bulm} \times 2 \geq \frac{100 \text{ kg} \times [40 \text{ cm}^2 \times (22 \text{ cm} - 10 \text{ cm} - 6,28 \text{ cm})]}{3835,06 \text{ cm}^4} \cdot \frac{1}{6} \cdot 20 \text{ cm}$$

$$\underbrace{\hspace{15em}}_{5,97 \text{ kg/cm}^2}$$

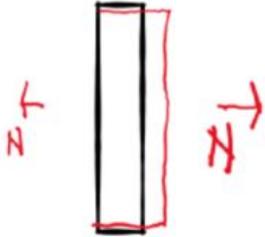
$$\textcircled{2} A_{bulm} = 5,97 \text{ cm}^2$$

$d_{bulm} \geq 2,76 \text{ cm} \rightarrow$ debería adicionar la separación y/o usar bulones + resistentes

Deformación por flexión

Deformaciones x Flexión y corte

AXIL



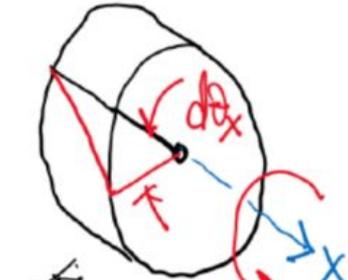
$$\frac{du}{dx}$$

$$\epsilon_x = \frac{du}{dx}$$

$$\sigma = E \cdot \epsilon_x$$

$$\epsilon_x = \frac{N}{EA}$$

Torsion

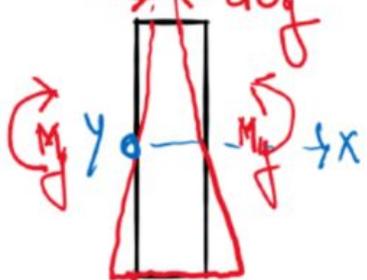


$$\frac{d\theta_x}{dx}$$

$$\gamma_{xy} = \frac{d\theta_x}{dx}$$

$$d\theta_x = \frac{M_t}{G \cdot J_p} \cdot dx$$

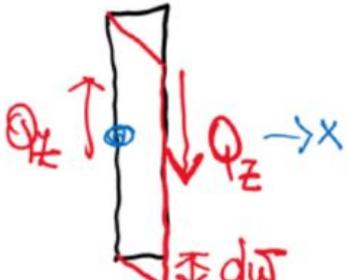
FLEXIÓN



$$\frac{d\alpha_y}{dx}$$

$$d\alpha_y = \frac{M_y}{E \cdot J_y} \cdot dx$$

COORTE



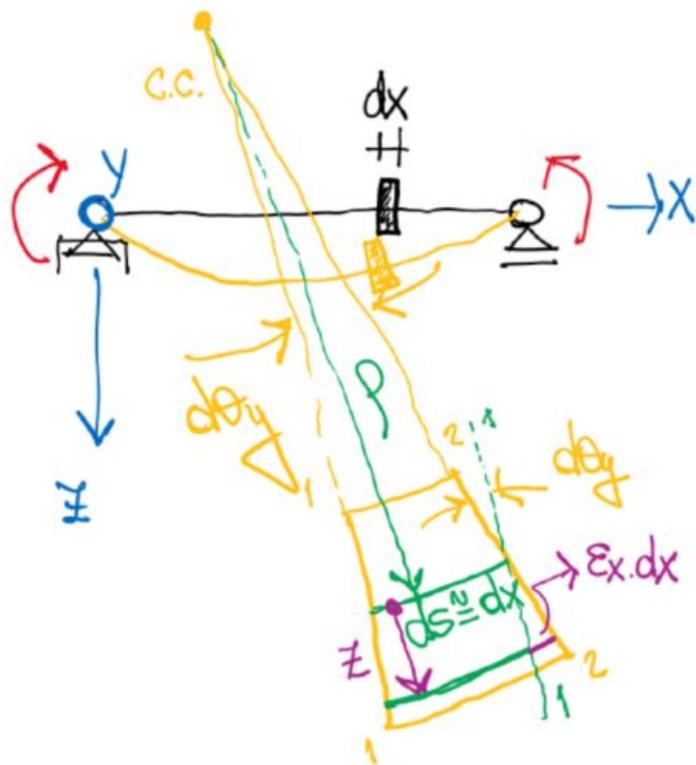
$$\frac{d\omega}{dx}$$

$$d\omega = \frac{Q_z}{G \cdot A} \cdot k_z \cdot dx$$

$$A_R = \frac{A}{k_z}$$

se deforma conservando la dirección del eje
C.C.

se deforma en la dirección transversal a la eje de barra - EJE CURVO



EXPRESION MATEMATICA DE LA CURVATURA

$$\rho = \frac{1}{\kappa} \rightarrow \kappa_y = \frac{d\theta_y}{dx} = \frac{1}{\rho}$$

$$\rho \cdot d\theta_y = dx$$

$$\epsilon \cdot d\theta_y = \epsilon_x \cdot dx \rightarrow \epsilon_x = \kappa_y \cdot \epsilon$$

$$\sigma_x = E \epsilon_x = E \kappa_y \cdot \epsilon$$

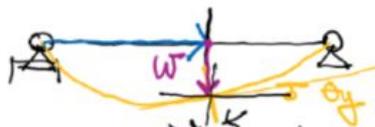
$$\sigma_x = \frac{M_y}{I_y} \cdot \epsilon = E \kappa_y \cdot \epsilon \Rightarrow \kappa_y = \frac{M_y}{E I_y}$$

$$\kappa = \frac{d^2 w / dx^2}{\left[1 + \left(\frac{dw}{dx}\right)^2\right]^{3/2}}$$

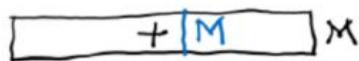
$$\frac{dw}{dx} = w' \lll 1 \rightarrow w' = 0$$

Ejercicio 1

$$\chi_y = \frac{d\omega^2}{dx^2}$$



$$\chi_y = \frac{d\theta_y}{dx} = \frac{d\omega}{dx^2} = -\frac{M}{E \cdot J_y}$$



CONDICIONES DE BORDE

$$\omega' = -\frac{M}{E J_y} \cdot x + C_1$$

$$\omega = -\frac{M}{E J_y} \cdot \frac{x^2}{2} + C_1 \cdot x + C_2$$

$$x=0 \rightarrow \omega(0)=0 \rightarrow C_2=0$$

$$x=L \rightarrow \omega(L)=0 \rightarrow C_1 = \frac{M \cdot L}{2 E J_y}$$

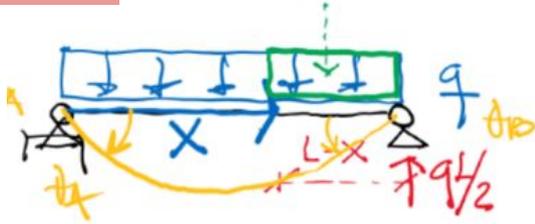
$$\omega = -\frac{M}{E J_y} \cdot \frac{x^2}{2} + \frac{M \cdot L}{2 E J_y} \cdot x$$

$$\omega_{max} \rightarrow \frac{d\omega}{dx} = 0 \rightarrow x = \frac{L}{2}$$

$$\omega_{max} = \frac{M \cdot L^2}{8 E \cdot J_y}$$

Ejercicio 2

V.S.A. con carga distribuida uniforme



$$M(x) = \frac{qL}{2}(L-x) - q\frac{(L-x)^2}{2}$$

$$M(x) = \frac{q}{2}(Lx - x^2)$$

$$w(x) = \frac{q}{24EI_y} x^4 - \frac{q}{12EI_y} Lx^3 + \frac{q \cdot x \cdot L^3}{24EI_y}$$

$$w_{\max}(L/2) = \frac{5}{384} \frac{q \cdot L^4}{EI_y} \ll \frac{L}{200} \text{ o } \frac{L}{500}$$

$$\frac{d^2 w}{dx^2} = - \frac{M}{EI_y} = - \frac{q}{2} (Lx - x^2)$$

$$w'(x) = - \frac{q}{2EI_y} \left(L \frac{x^2}{2} - \frac{x^3}{3} \right) + C_1$$

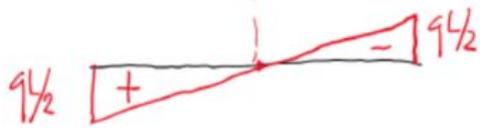
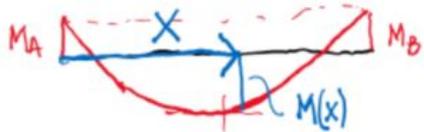
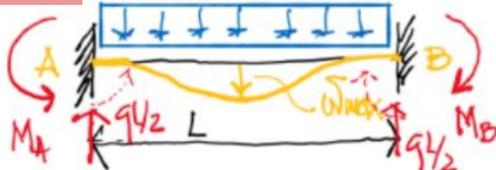
$$w(x) = - \frac{q}{24EI_y} \left(L \frac{x^3}{6} - \frac{x^4}{12} \right) + C_1 x + C_2$$

$$x=0 \rightarrow w(0) = 0 \rightarrow C_2 = 0$$

$$x=L \rightarrow w(L) = 0$$

Ejercicio 3

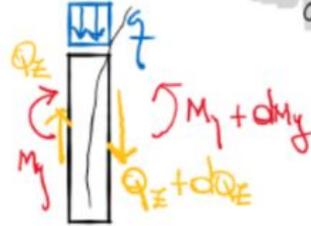
Ej 3 viga doblemente empotrada con una carga uniformemente distribuida



CONDICIONES DE BORDE

$$\begin{aligned}
 X=0 &\rightarrow w(0)=0 \rightarrow w'(0)=0 \\
 X=L &\rightarrow w(L)=0 \rightarrow w'(L)=0
 \end{aligned}$$

CARGA



CORTE

$$w^{IV} = + \frac{q}{EI} \cdot x + C_1$$

MOMENTO

$$w^{III} = + \frac{q}{EI} \cdot \frac{x^2}{2} + C_1 \cdot x + C_2$$

GIRO

$$w^{II} = + \frac{q}{EI} \cdot \frac{x^3}{6} + \frac{q}{2} x^2 + C_2 x + C_3$$

ELASTICIDAD

$$w = + \frac{q}{EI} \cdot \frac{x^4}{24} + \frac{q}{6} x^3 + \frac{C_2}{2} x^2 + C_3 x + C_4$$

Polinomio de 4to grado

$$\begin{aligned}
 w'' &= - \frac{M}{EI} \\
 w''' &= - \frac{dM}{dx} \cdot \frac{1}{EI} = - \frac{Q}{EI} \\
 w^{IV} &= - \frac{dQ}{dx} \cdot \frac{1}{EI} = + \frac{q}{EI} \\
 \frac{dM}{dx} &= Q \\
 dQ &= -q dx
 \end{aligned}$$

$$x = L/2 \rightarrow w'(L/2) = 0$$

$$x = L/2 \rightarrow w'''(L/2) = 0$$

$$w_{max} = \frac{1}{384} \frac{q L^4}{E J_y}$$

Ejercicio 4

Ej 4 viga empotrada / Simple apoyada con carga uniforme



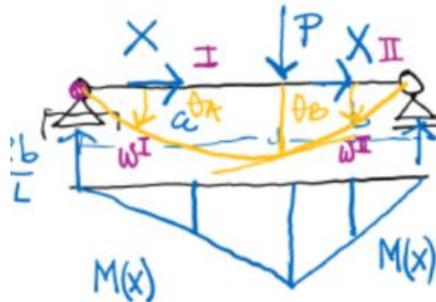
$$w(0) = 0$$

$$w(L) = 0$$

$$w''(L) = 0$$

$$w'(0) = 0$$

Ej 5 VSA con una concentrada



$$0 \leq x \leq a$$

$$a \leq x \leq L$$

$$M(x) = \frac{P \cdot a}{L} \cdot x \rightarrow w^I \begin{cases} C_1 \\ C_2 \end{cases}$$

$$M(x) = \frac{P \cdot a}{L} (L - x) \rightarrow w^{II} \begin{cases} C_3 \\ C_4 \end{cases}$$

$$w^I(0) = 0 \quad w^{II}(L) = 0$$

$$w^I(a) = w^{II}(a)$$

$$w^{I'}(a) = w^{II'}(a)$$

CONDICIONES DE BORDE

$$w''(0) = 0$$

$$w''(L) = 0$$

$$\frac{d^2 w}{dx^2} = - \frac{M(x)}{E J_y}$$