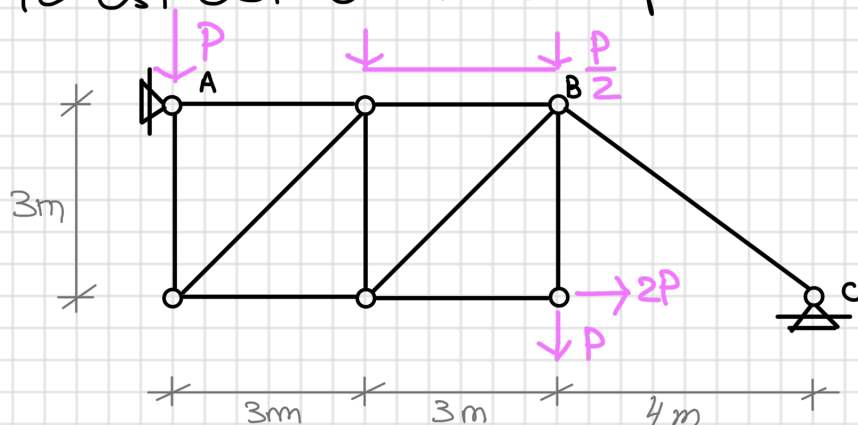


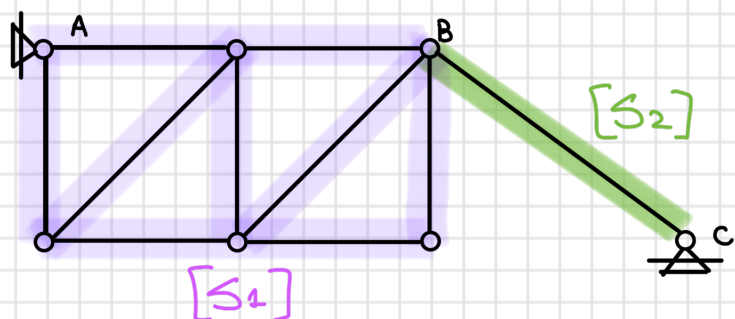
2Q25 egs !!

ESTRUCTURAS RETICULADAS

La siguiente estructura falla cuando algún elemento está trabajando a 25 kN axialmente. Halle el valor de P tal que la estructura no colapse



ANÁLISIS CINEMÁTICO



Retiwlado:

- ✓ forma triángulos
- ✓ barras rectas
- ✓ $\#B = 2 \times N - 3$
- $9 = 2 \times 6 - 3 = 12 - 3$
- $9 = 9$
- x Cargas en nodos
- ↓
- idealizaremos

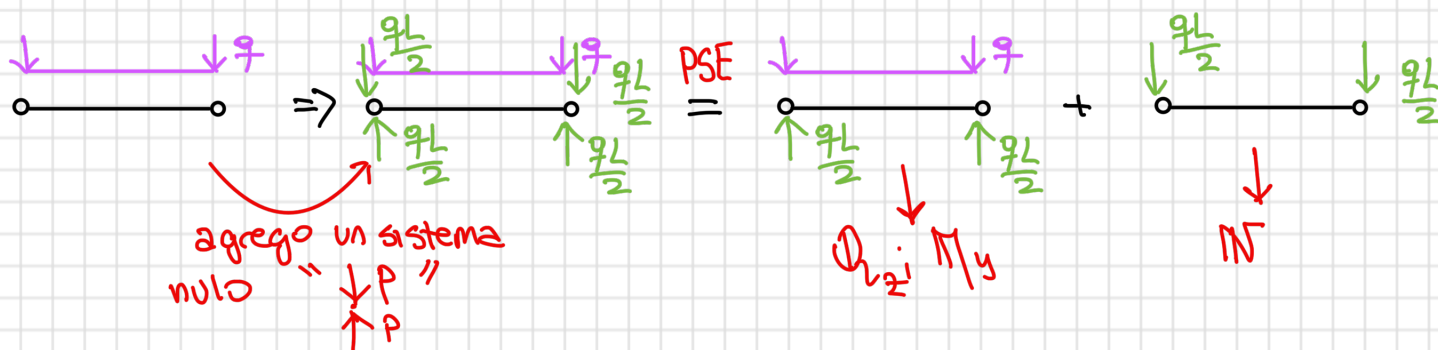
cadena abierta de dos chapas: $[S_1]$ y $[S_2]$ con PF en A y C respectivamente, articuladas con B; se forma un arco triarticulado. OBS.
 ✓ recta que contenga a A, B y C en simultánea. $CV = CL = 4$ ✓

IDEALIZACIÓN DEL RETICULADO [PSE]

ESTÁTICA (nuestra materia) → valen las tres linealidades

- LM linealidad mecánica
- LC linealidad cinemática → vale
- LE linealidad estática

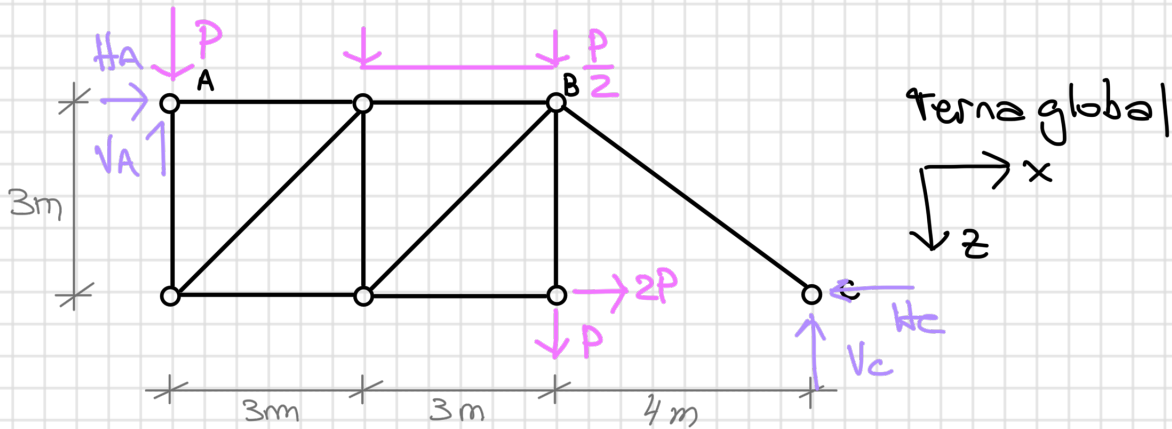
PSE



efs

Si $q = \frac{P}{2}$ y $L = 3m \Rightarrow \frac{qL}{2} = \frac{(\frac{P}{2})3}{2} = \frac{3P}{4}$

CÁLCULO RVE

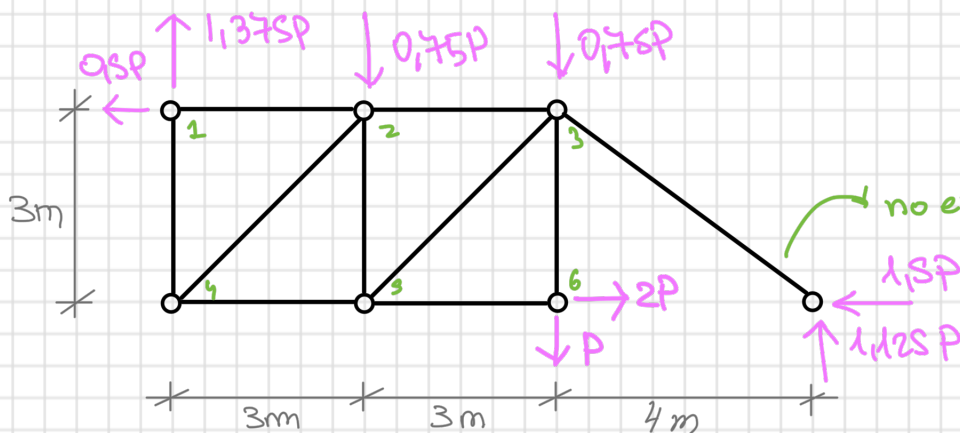
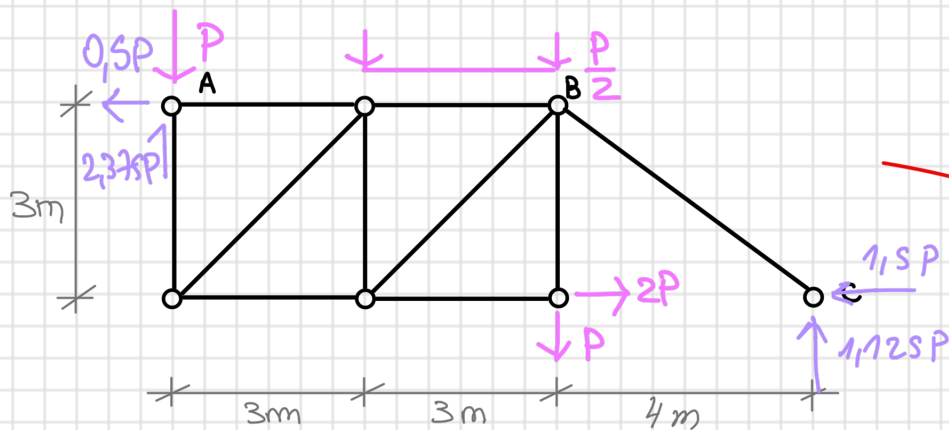


$$\sum M_{\text{rect}}^B = -V_A 6m + 2P 3m + \left(\frac{P}{2} 3m\right) 3m/2 + P 6m = 0 \rightarrow V_A = 2,375 P$$

$$\sum F_V = -V_C - 2,375 P + P + P + \frac{P}{2} 3m = 0 \rightarrow V_C = 1,125 P$$

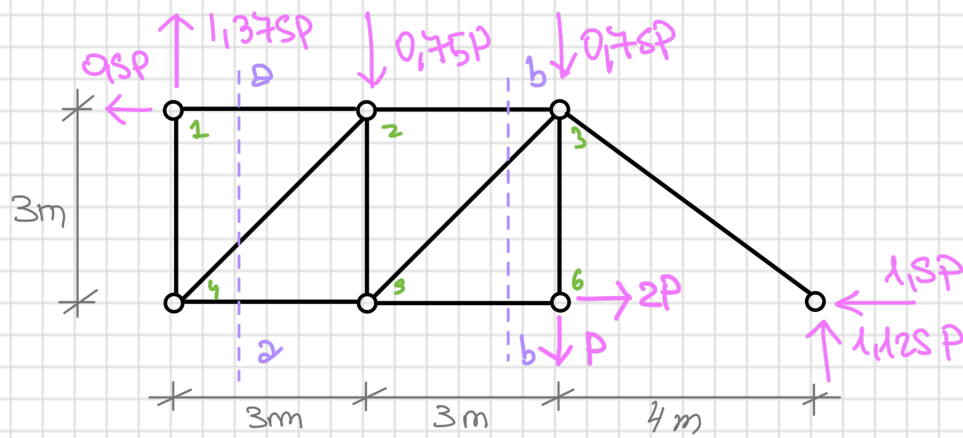
$$\sum M^C = P 4m + \left(\frac{P}{2} 3m\right) \left(4m + \frac{3}{2}m\right) + P 10m - \underbrace{V_A 10m}_{2375P} - H_A 3m = 0 \rightarrow H_A = -95P$$

$$\sum F_H = \underbrace{H_A}_{-0,5P} + 2P - H_C = 0 \rightarrow H_C = 1,5P$$



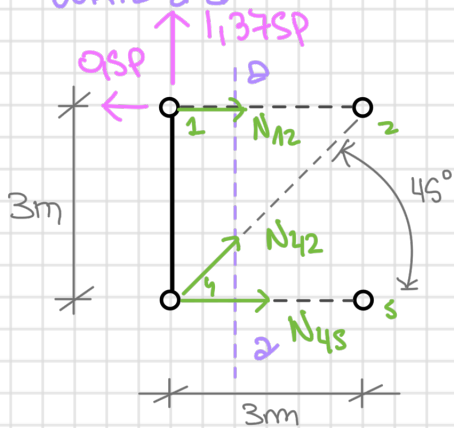
reescribo para hallar M

no es parte del reticulado pero como no tiene carga en toda la barra, será un punto (basta a compresión)



Supongo barras
traccionadas

CORTE a-a



$$\sum M^2 = N_{45} 3m - 1.375P 3m = 0$$

$$N_{45} = 1.375P$$

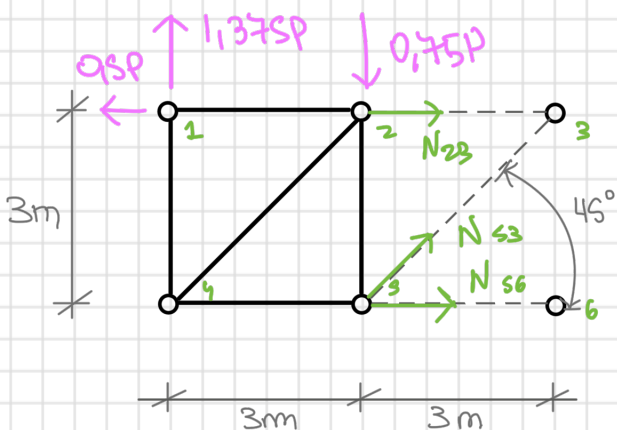
$$\sum M^4 = N_{45} 3m + N_{42} \cos 45^\circ 3m = 0$$

$$N_{42} = -1.94P$$

$$\sum M^1 = 0.5P 3m - N_{12} 3m = 0$$

$$N_{12} = 0.5P \rightarrow \text{sate rápido con el nodo ①.}$$

CORTE b-b



$$\sum M^5 = -N_{23} 3m - 1.375P 3m + 0.5P 3m = 0$$

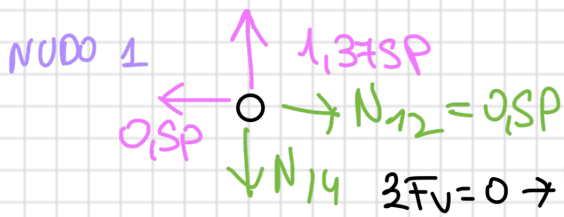
$$N_{23} = -0.875P$$

$$\sum M^2 = N_{53} \cos 45^\circ 3m + 2P 3m - 1.375P 3m = 0$$

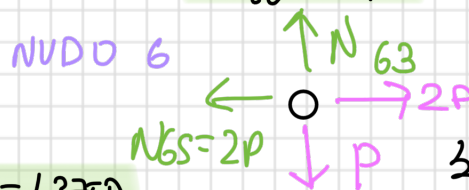
$$N_{53} = -0.884P$$

$$\sum M^3 = +0.75P 3m - 1.375P 6m + N_{56} 3m = 0$$

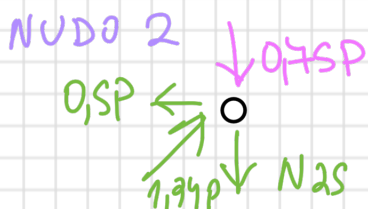
$$N_{56} = 2P$$



$$\sum F_V = 0 \rightarrow N_{14} = 1.375P$$



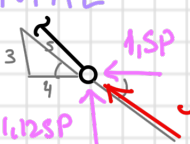
$$\sum F_H = 0 \rightarrow N_{63} = P$$



$$\sum F_V = 0 = N_{25} + 0.75P - 1.94P \sin 45^\circ = 0$$

$$N_{25} = 0.622P$$

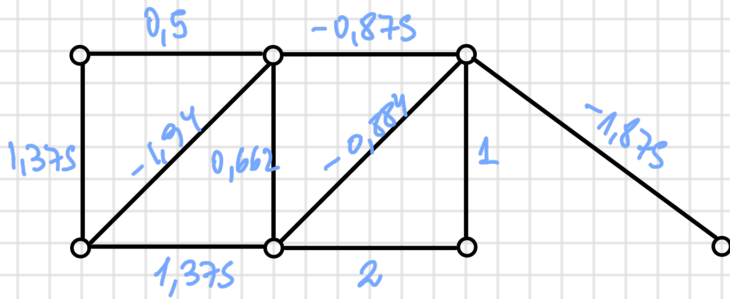
PUNTA



$$N_{\text{Punta}} = \sqrt{(1.5P)^2 + (1.125P)^2} = 1.875P \ominus$$

solo IN
solo vale esto pues no tiene cargas a lo largo de la barra

$N [P]$



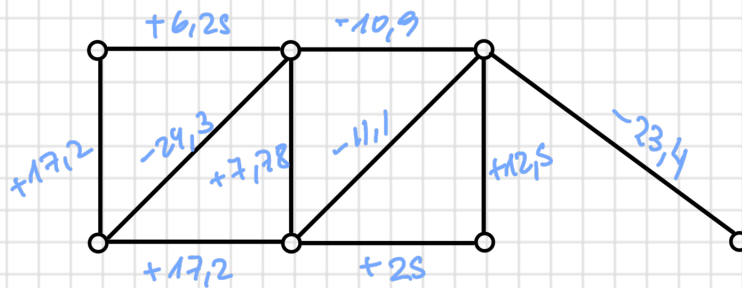
Por enunciado, falla cuando algún elemento tiene $N = 25 \text{ kN}$
 barra más solicitada $\rightarrow$ mayor riesgo de fallar primero
 barra "56" $\rightarrow N_{56} = 2P$

$$25 \text{ kN} = 2P$$

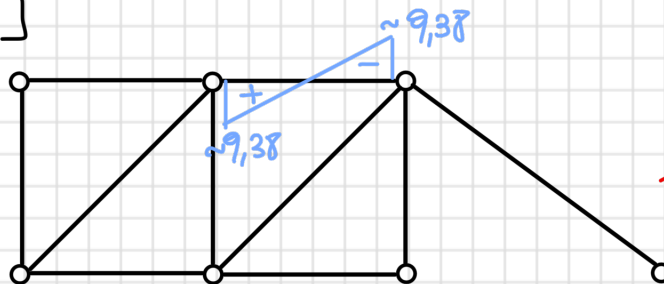
$$P = 12,5 \text{ P}$$

¡falta resto de diagramas! $\checkmark$ si no, se cae. (o nos bochan)
 (Q_y y M)

$N [kN]$



$Q_z [kN]$



no olvidarse del Q_y
 M al "idealizar."

$M_y [kNm]$

